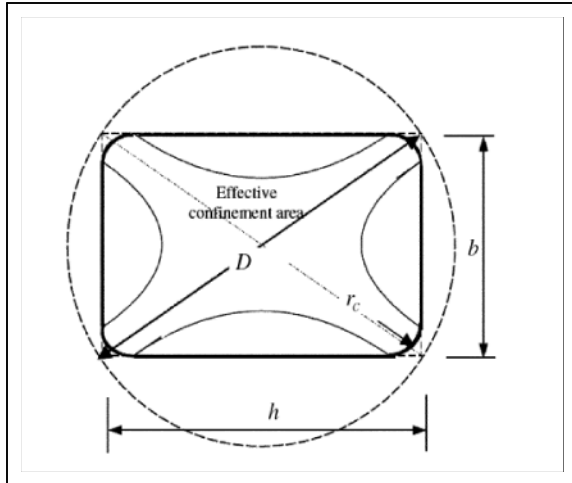


this spreadsheet is intended to calculate the net axial confinement strengthening of FRP wraps applied to bridge columns. Reference document ACI 440.2R-08. Use of FRP wraps for axial confinement strengthening is not recommended for members featuring side aspect ratios  $h/b$  greater than 2.0, or face dimensions  $b$  or  $h$  exceeding 36 in., unless testing demonstrates their effectiveness (ACI 12.1.2).

definition section - information from manufacturer's data sheets and bridge plans

Column Dimensions (Figure 12-3 from ACI 440.2R-08)



ColumnType :=



ColumnType = "Rectangular"

Inputs

FRP

$\epsilon_f := 0.0167$

Fiber ultimate strain (in/in), provided by manufacturer

$E_f := 33000000\text{psi}$

Fiber tensile modulus (psi), provided by manufacturer

$n := 1$

number of FRP wraps, typically 2 but modified as needed

$t_f := 0.0065\text{in}$

FRP thickness (in), provided by manufacturer

Concrete

$$f'_c := 3500 \text{ psi}$$

concrete compressive strength (psi), substructure concrete MDOT Grade S2 is 3500 psi.

$$\rho_g := 0.02$$

longitudinal steel reinforcement ratio assumed at 0.02

$$\epsilon'_c := 0.002$$

maximum strain of unconfined concrete corresponding to  $f'_c$ , in./in; may be taken as 0.002

Column

$$h := 60 \text{ in}$$

Input h and b dimensions. leave as default if circular column.

$$b := 48 \text{ in}$$

$$r_c := 3 \text{ in}$$

corner radius typically 3 in, but manufacturer recommended value

$$D_c := 42 \text{ in}$$

input the circular column dimension. leave as default if rectangular column.

$$D := \begin{cases} \sqrt{b^2 + h^2} & \text{if ColumnType} = \text{"Rectangular"} \\ D_c & \text{otherwise} \end{cases} \quad \text{ACI equation 12-8 for equivalent circular cross section}$$

$$D = 76.837 \text{ in}$$

$$A_g := \begin{cases} b \cdot h & \text{if ColumnType} = \text{"Rectangular"} \\ \frac{\pi \cdot D^2}{4} & \text{otherwise} \end{cases} \quad \text{Gross cross section area}$$

$$A_g = 2880 \text{ in}^2$$

effective concrete confinement cross sectional area ratio ( $A_e/A_c$ ) for non-circular shapes

this ratio is used for non circular cross sections to calculate the appropriate shape factors,  $\kappa_a$  and  $\kappa_b$ , which depend on the area of effectively confined concrete and the side-aspect ratio h/b.

See ACI section 12.1.2.

$$\text{EffectiveConfinementRatio} := \frac{\left[ 1 - \frac{\left[ \frac{b}{h} \cdot (h - 2r_c)^2 + \frac{h}{b} \cdot (b - 2r_c)^2 \right]}{3A_g} - \rho_g \right]}{1 - \rho_g} \quad \text{ACI equation 12-11}$$

EffectiveConfinementRatio = 0.464

Shape Factors (non circular shapes). For circular shapes, ACI 12.1.2 specifies  $\kappa_a$  and  $\kappa_b$  set to

$$\kappa_a := \begin{cases} \text{EffectiveConfinementRatio} \cdot \left(\frac{b}{h}\right)^2 & \text{if ColumnType} = \text{"Rectangular"} \\ 1.0 & \text{otherwise} \end{cases} \quad \text{ACI Equation 12-9}$$

$$\kappa_a = 0.297$$

$$\kappa_b := \begin{cases} \text{EffectiveConfinementRatio} \cdot \left(\frac{h}{b}\right)^{0.5} & \text{if ColumnType} = \text{"Rectangular"} \\ 1.0 & \text{otherwise} \end{cases} \quad \text{ACI Equation 12-10}$$

$$\kappa_b = 0.519$$

FRP Strain efficiency factor,  $\kappa_\epsilon$

$$\kappa_\epsilon := 0.55$$

Per ACI section 12.1

FRP effective strain at failure,  $\epsilon_{fe}$

$$\epsilon_{fu} := 0.85 \cdot \epsilon_f$$

FRP ultimate strain reduced by environmental factor of 0.85 per ACI table 9-1

$$\epsilon_{fe} := \begin{cases} \kappa_\epsilon \cdot \epsilon_{fu} & \text{if } \kappa_\epsilon \cdot \epsilon_{fu} < 0.004 \\ 0.004 & \text{otherwise} \end{cases}$$

ACI Equation 12-12. Limited to ensure the shear integrity of the confined concrete.

$$\epsilon_{fe} = 0.004$$

Maximum compressive strain in the confined concrete,  $\epsilon_{ccu}$

$$f_l := \frac{2E_f \cdot n \cdot t_f \cdot \epsilon_{fe}}{D} \quad \text{ACI equation 12-4}$$

$$f_l = 22.333 \text{ psi}$$

$$f'_{cc} := f'_c + 0.95 \cdot 3.3 \cdot \kappa_a \cdot f_l \quad \text{ACI equation 12-3}$$

$$f'_{cc} = 3521 \text{ psi}$$

$$\epsilon_{ccu} := \epsilon'_c \cdot \left[ 1.50 + 12\kappa_b \cdot \frac{f_l}{f'_c} \left( \frac{\epsilon_{fe}}{\epsilon'_c} \right)^{0.45} \right]$$

$$\epsilon_{ccu} = 0.003$$

Note that  $\epsilon_{ccu}$  must be  $\leq 0.01$  per ACI equation 12-7.

$$\text{ConfinementRatio} := \frac{f_l}{f'_c}$$

$$\text{ConfinementRatio} = 0.006$$

ACI recommends the confinement ratio to be  $\geq$  0.08

Additional strength contribution of the FRP wrap

$$\phi := 0.65$$

axial compression strength reduction factor,  $\Phi$  per ACI 318

$$P_{\text{net}} := 0.8 \cdot \phi \cdot \left[ 0.85 (f'_{cc} - f'_c) \cdot A_g \cdot (1 - \rho_g) \right]$$

Modified from ACI equation 12-1b, to determine the net axial strength contribution of FRP

$$P_{\text{net}} = 25.9 \cdot \text{kip}$$

