



Furthermore, two values were used to compare the performance of bridges with FDPC deck panel decks to those with CIP decks: (a) deterioration rate of the deck and (b) estimated service life.

### Deterioration Rate and Estimated Service Life

The calculated variables used to compare the performance of the bridges were deterioration rate and estimated service life. The deterioration rate is the slope of the linear regression of the year and deck rating since the time of deck construction. The estimated service life of the deck was calculated based on the time it takes for the deck rating to reach a rating of 4, based on the deterioration rate and the starting deck rating. A deck rating of 4 was used as the threshold for deck repair needed as this value corresponds to the boundary between fair and poor behavior used by previous researchers [5].

### Ranking of Comparison Projects

All comparison projects needed to have the same of the following parameters to be considered valid comparisons:

- Material and structure type
- Overlay or wearing surface
- Climate zone

Both comparison projects needed to either have an overlay or wearing surface or not have an overlay or wearing surface. The type of overlay did not need to be the same for the comparison projects.

Other variables could have different values between them. The quality of the comparison was rated based on how similar the comparison projects were with these other criteria. These variables included:

- Span length
- Year of construction
- ADT/ADTT

A rating was given to each comparison based on the degree of similarity of the values for each of the comparison projects.

## RESULTS

The results of this study are summarized in two categories:

### Side-by-Side Comparison

A side-by-side comparison was conducted for the 280 comparison projects from entire FDPC Deck Panel Database. In 49 of the 172 comparison projects, the bridge with a CIP deck had a higher deterioration rate, shown in Figure 2. The bridge with a FDPC deck panel deck had a higher deterioration rate in 76 of the 173 comparison projects. The bridges with the two types of decks had the same deterioration rates in 47 of the comparisons.

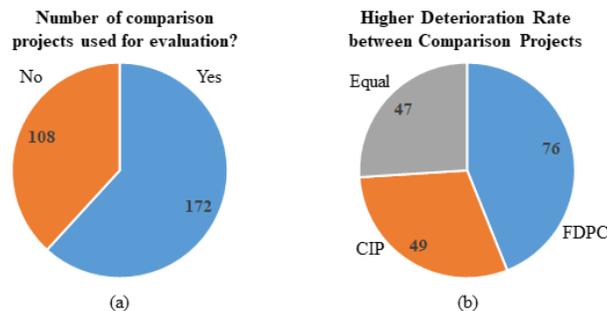


Figure 2: (a) Number of comparison projects used for evaluation (b) type of bridge with higher deterioration rate

Bridges with FDPC deck panels were found to have a similar performance (similar deterioration rate and estimated service life) to the comparison bridges with CIP decks. Table 1 summarizes the overall comparison of FDPC deck panel decks and CIP decks

Table 1: Overall average performance of bridges with FDPC deck panel decks and CIP decks

Deck Type	FDPC	CIP
<i>n</i> <sub>bridges</sub>	206	177
Avg. <i>n</i> <sub>inspections per bridge</sub>	12.5	13.0
Avg. Year of 1 <sup>st</sup> Inspection	2004	2005
Deterioration Rate	-0.12	-0.09
Estimated Service Life (year)	33	35

### Performance Comparison Based on the Classification of Variables

Important variables that were used for comparison purpose includes: joint type, impact category, climate zone, wearing surface, main span material type and traffic volume.

### CONCLUSION

The performance of FDPC deck panel and CIP systems was evaluated based on the NBI deck rating. The average performance of bridges with FDPC deck panels were compared with similar bridges with CIP decks. Results showed almost similar performance for both bridges with FDPC deck panel and CIP decks. As the precast panel itself offers superior durability to CIP decks, these results may suggest that there is room for improvement with joint design and construction.

These performance comparisons were further evaluated in several subcategories. Focusing on joint types revealed that UHPC with straight bar (for transverse or longitudinal joints), longitudinal post-tensioned (for transverse joints), and conventional concrete with hooped bar details (for longitudinal joints) are most popular and have good performance. Considering climate condition showed that shortest average estimated service life is observed in cold climate zones due to a lot of freeze-thaw cycles, which combined with moisture and deicing salts. Also, results showed that all overlay types and bridges without overlays perform almost similarly. There are limitations to this approach, but this work can be used as a starting point to a more in-depth evaluation of these projects (e.g. non-destructive testing) and revisited every 5 to 10 years as the bridges continue to age and new bridges are constructed.

### REFERENCES

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