## Non-Proprietary UHPC Workshop

## Consideration for Local Materials

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## Outline

- Background
- Mix Design and General Overview
- Basic Steps for Non-Proprietary UHPC Mix Development
- Small-Batch Results
- Large-Batch Results
- Conclusions and Recommendations





## Background

Ultra-High Performance Concrete (UHPC)

- Cementitious composite material with high compressive strength, stiffness, and tensile strength and sustained post-cracking tensile strength and strain hardening response
- Steel fibers are typically included in the mixture to provide the required post-cracking response
- Pre-bagged commercial (or proprietary) UHPC products and nonproprietary UHPC mixtures are both available





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## Background

Ultra-High Performance Concrete (UHPC)

Property	Typical Range
7-day Compressive Strength	14.5 to 19.5 ksi
14-day Compressive Strength	18 to 22 ksi
Direct Tensile Cracking Strength	0.8 to 1.2 ksi
Direct Tension Bond Test	0.35 to 0.6 ksi
Modulus of Elasticity	4,250 to 8,000 ksi
Long-term Drying Shrinkage	300 to 1,200 με
Long-term Autogenous Shrinkage	200 to 900 με
Initial setting time	4 to 10 hours
Final setting time	7 to 24 hours
Static flow	7.5 to 10 inches



B. Graybeal, "Design and construction of field-cast UHPC Connections," Federal Highway Administration (FHWA), FHWA-HRT-19-011, 2019



## Background

Non-Proprietary UHPC

- UHPC typically consists of:
  - Binders / SCMs
    - Cement
    - · Silica fume
    - Other SCMs (e.g., fly ash, slag)
  - Fine Aggregate
    - Silica Sand / Crushed Limestone
    - Ground Quartz
  - Chemical Admixtures
    - Superplasticizer / High-Range Water Reducer (HRWR)
    - Viscosity Modifying Admixture (VMA)
  - Fibers (typically steel with 13mm length and 0.2mm diameter)



**Supplementary Cementitious** 









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## Background

Non-Proprietary UHPC – Sample of Previous Studies

				Sele	cted-UHPC Mix P	arameters			Perform	ance
Researcher	Year	Location	c: SF: SCM	Other SCMs Used	w/c	w/b	Agg.:b	Fiber vol. fraction (%)	Flow (in.)	f <sub>c</sub> (ksi)
		А	1.0: 0.25: 0.00	-	0.25	0.200	0.88	0 and 2	8-11	25.0
		В	1.0: 0.25: 0.11	LP	0.25	0.184	1.10	0 and 2	8.9, 9.2	23.4
Tadros et al.1	2020	С	1.0: 0.25: 0.00	-	0.24	0.195	0.77	0 and 2	9.1	23.1
		D	1.0: 0.20: 0.18	LP	0.29	0.202	0.77	0 and 2	9.1	21.4
		Е	1.0: 0.25: 0.00	-	0.23	0.188	1.10	0 and 2	8.9	23.6
Lawler et al.	2019	FL	1.0: 0.15: 0.15	FA (Class F)	0.23	0.170	1:0 to 2:0	1.5 and 2	8-10	18-19
Karim et al. <sup>2</sup>	2019	lowa	1.0: 0.07: 0.00	-	0.20, 0.25	0.18, 0.2, 0.23	1.12, 1.3	2	8-9	10-17
Matos et al.	2019	Portugal	1.0: 0.54: 0.27	-	0.40	-	1.0	3	11.2-12.2	21-22
Looney et al.	2019	ОК	1.0: 0.17: 0.50	S	0.18 to 0.22	0.18 to 0.23	0.75, 1.0	1 and 2	9-11	16-18.2
Berry et al.	2017	Montana	SF/FA = 0.75	FA	0.24	-	1.4 <sup>3</sup>	0 and 2	8-11	20-21
El-Tawil et al.	2016	Michigan	1.0: 0.25: 1.0	S	0.22	0.18	1.0	1.5	-	20.9- 28.3
Graybeal	2013	WA, OR, ND, SD, NY, PA	1.0: 0.25: 0.25	FA	0.22 to 0.24	0.15 to 0.16	1.0	1 and 2	10.4-12.4	22.5-29
Tafraoui et al.	2009	France	1.0: 0.25: 0.25	Metakaolin	0.27	0.22	0.9, 1.18	0 and 2	-	15-27.5

FLORIDA INTERNATIONA UNIVERSITY c = cement; SF = silica fume; b = binders = all cementitious materials; FA = fly ash; LP = limestone powder; S = slag or GGBS

¹liquid portion of chemical admixtures was included in w.c and w.b calculations; ²compressive strength was measured at 7 days; ² this is sand to cement ratio

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# Basic Steps for Non-Proprietary UHPC Mix Development

- 1. Survey locally available materials
  - Fine aggregate, Cement, Source of other SCMs (e.g., slag, fly ash, silica fume)
- 2. Select previous research with similar types of available materials (can start by looking for study closest to you) to use as a starting point
- 3. Measure particle size distributions for available materials (our local cement/aggregate producer did this for us)
- 4. Determine appropriate proportions of materials to fall closest to the ideal curve
- 5. Determine compression strength of several different options using small-batch mixtures; will also get a sense of needed HRWR in small batches
- Test additional material properties (e.g., MOR, direct tension, durability-related properties) and scalability with large-batch mixture for best performing mixture





South Florida

materials

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## Available Materials Investigated

- Aggregate: Masonry Sand TITAN (Miami)
- Cement: Type I/II, III, Masonry TITAN (Miami)
- Slag: ARGOS USA (Tampa)
- **Superplasticizer**: Glenium 7920 BASF
- VMA: MasterMatrix VMA 358 BASF
- Silica fume BASF
- Steel Fibers: Hiper Fiber Type A, Bekaert OL 13/.20, Dramix 4D 65/35BG, Helix 5-13
- Synthetic Fibers: GCP STRUX® 90/40





## Available Steel Fibers Investigated









**DRAMIX 4D 65/35BG** 

Helix 5-13

Hiper Fiber Type A

GCP STRUX® 90/40

Length: Diameter: 35mm (1.4") 0.55mm (0.02") 13mm (0.5") 0.5mm (0.02") 26 13mm (0.5") 0.2mm (0.008") 65 40mm (1.55") 0.43mm (0.017") 92

Aspect Ratio: Tensile Strength: 65 1,850 MPa (268.0 ksi)

1,700 MPa (246.5 ksi)

2,758 MPa (400.0 ksi)

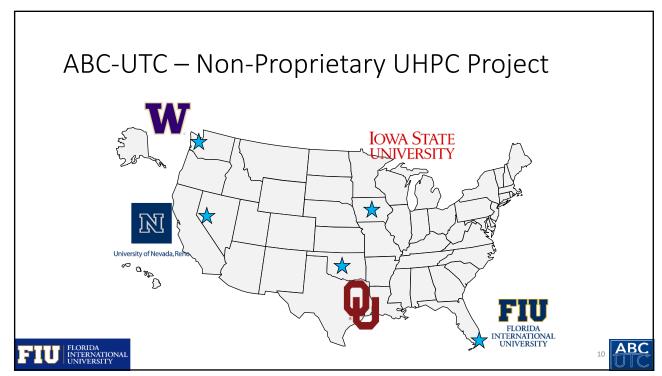
620 MPa (90.0 ksi)



(high tensile strength so fibers slip before rupturing, gives ductility)

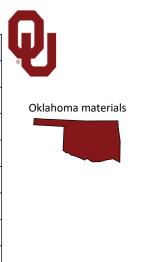


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## Base Mix Design

Component	Quantity
Type I Cement, lb/yd³	1179.6
Slag, lb/yd <sup>3</sup>	589.8
Silica Fume, lb/yd³	196.6
w/cm	0.23
Fine Masonry Sand, lb/yd <sup>3</sup>	1966
Steel Fibers, lb/yd³	255.2
Steel Fibers, %	2.0
Glenium 7920, oz./cwt	15.77

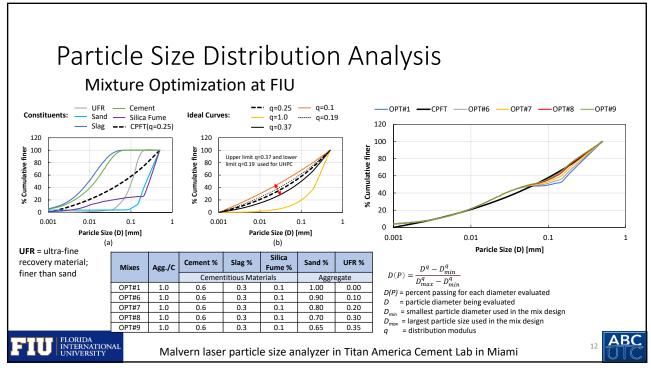


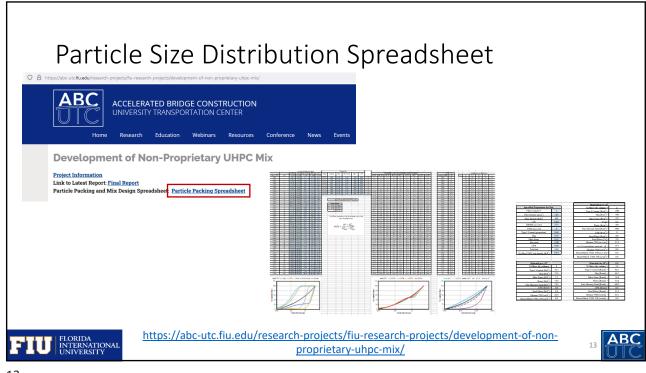


Starting point for FIU mix design



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## Determining Amount of Material for Mixtures

#### **Proportions for Mixtures**

Constituent	Proportion	Variable
agg/cm	1.0	
Cement	0.6	$P_c$
Slag	0.3	P <sub>s</sub>
Silica Fume	0.1	P <sub>sf</sub>
Fine Sand	0.9	P <sub>sand</sub>
UFR	0.1	P <sub>UFR</sub>
Total Units	2.0	P <sub>tot</sub>

The total units here should equal 2.0. We found 1.0:1.0 binder-to-aggregate ratio to be best.

#### **Other Information Needed**

Property	Value	Variable
Fiber Content [%]	2.0	FC
Fiber Density [lb/ft³]	490	$ ho_f$
Water-to-binder ratio	0.2	w/b
HRWR [oz./cwt]	27.5	V <sub>HRWR</sub>
VMA [oz./cwt]	0.0	$V_{VMA}$
Estimated Density [lb/ft³]	148.6	$ ho_c$

Value shown here was measured on a mix with 0% fibers. 150 lb/ft<sup>3</sup> is a good initial estimate.



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## Determining Amount of Material for Mixtures

• Cement:

$$W_c = \frac{\rho_c P_c (1 - FC)}{P_{tot}} = \frac{\left(148.6 \ lb/f_{t^3}\right) (0.6)(1 - 0.02)}{2.0} = 43.7 \ lb/f_{t^3}$$

• Slag:

$$W_s = \frac{\rho_c P_s (1 - FC)}{P_{tot}} = \frac{\left(148.6 \ lb/f_{t^3}\right) (0.3)(1 - 0.02)}{2.0} = 21.8 \ lb/f_{t^3}$$

· Silica Fume:

$$W_{sf} = \frac{\rho_c P_{sf} (1 - FC)}{P_{tot}} = \frac{\left(148.6 \ lb/f_{t^3}\right) (0.1)(1 - 0.02)}{2.0} = 7.3 \ lb/f_{t^3}$$

• Water:

$$W_w = (W_c + W_s + W_{sf})(w/b) = (43.7 lb/f_{t^3} + 21.8 lb/f_{t^3} + 7.3 lb/f_{t^3})(0.2) = 14.6 lb/f_{t^3}$$





## Determining Amount of Material for Mixtures

The amount of fine sand and UFR can be found using a similar procedure as the cementitious materials

$$W_{sand} = \frac{\rho_c P_{sand} (1 - FC)}{P_{tot}}$$

$$W_{UFR} = \frac{\rho_c P_{UFR} (1 - FC)}{P_{tot}}$$

• Fibers:

$$W_{fibers} = \rho_f(FC) = \left(490 \ lb/_{ft^3}\right)(0.02) = 9.8 \ lb/_{ft^3}$$

• HRWR:

$$V_{HRWR} = v_{HRWR} \frac{\left(W_c + W_s + W_{sf}\right)}{100} = \left(27.5 \frac{oz}{cwt}\right) \frac{\left(43.7 \frac{lb}{ft^3} + 21.8 \frac{lb}{ft^3} + 7.3 \frac{lb}{ft^3}\right)}{100 \, lb} = 20.0 \frac{oz}{ft^3}$$





## Determining Amount of Material for Mixtures

- Take these amounts per cubic foot times your total desired volume
- More details in Final Report

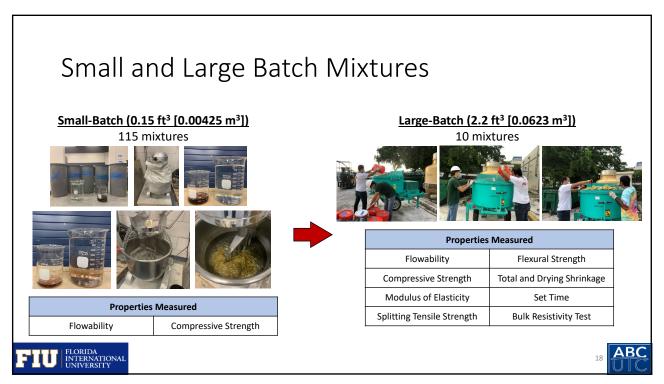
Shahrokhinasab and Garber (2021), Development of "ABC-UTC Non-Proprietary UHPC" Mix, Report No. ABC-UTC-2016-C2-FIU01-Final

Constituent	Amount per ft <sup>3</sup>	Amount per 0.15ft <sup>3</sup>
Cement [lb]	43.7	6.6
Slag [lb]	21.8	3.3
Silica Fume [lb]	7.3	1.1
Water [lb]	14.6	2.2
Fine Sand [lb]	65.5	9.8
UFR [lb]	7.3	1.1
Steel Fibers [lb]	9.8	1.5
HRWR [oz]	20.0	3.0
VMA [oz]	0.0	0.0





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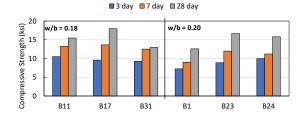


## Experimental Results – Small-Batch

#### Aggregate Moisture Content

Natural moisture content led to large variability in results. Aggregate should be oven dried for most consistent results.

	Cement Type			Mi	k Pro	porti	ions		Fiber		Admix	tures	Density	Sand
Mix.		w/b	agg/c	С	c	S SF	FA	UFR	Туре	Content	HRWR	VMA	(lb/ft³)	Moisture
			m		٦		1/4	OTK	1,700	(%)	(oz./cwt)	(oz./cwt)	(ID) It )	Wioisture
B11	Titan Type I/II	0.18	1.0	0.6	0.3	0.1	1.0	0.0	-	0.0	23.81	0.00	146.10	N
B17	Titan Type I/II	0.18	1.0	0.6	0.3	0.1	1.0	0.0	-	0.0	23.81	0.00	146.40	N
B31	Titan Type I/II	0.18	1.0	0.6	0.3	0.1	1.0	0.0	-	0.0	23.81	0.00	144.90	N
B1	Titan Type I/II	0.20	1.0	0.6	0.3	0.1	1.0	0.0	-	0.0	15.75	0.00	138.60	N
B23	Titan Type I/II	0.20	1.0	0.6	0.3	0.1	1.0	0.0	-	0.0	23.81	0.00	146.00	N
B24	Titan Type I/II	0.20	1.0	0.6	0.3	0.1	1.0	0.0	·	0.0	21.97	0.00	145.80	N



Aggregates were oven dried for all mixtures in Series C (small-batch) and all large batch mixtures



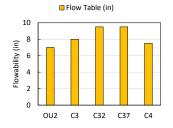
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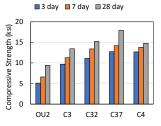
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# Experimental Results — Small-Batch Cement Type

Type I/II had high strength, good workability, and was the least expensive

Mix.	lix. Cement Type	w/b	Mix Proportions						Fiber		Admixt	ures	Density	Sand
			<b>b</b> agg/cm	С	S	SF	FA	UFR	Туре	Content	HRWR	VMA	(lb/ft³)	Moisture
				-						(%)	(oz./cwt)	(oz./cwt)		
OU2	Masonry Cement	0.20	1.0	0.6	0.3	0.1	1.0	0	Α	2.0	15.77	0	135.7	N
C3	Ash Grove Type I-II	0.20	1.0	0.6	0.3	0.1	1.0	0	OL	2.0	22.25	0	149.0	D
C32	Titan Type I/II	0.20	1.0	0.6	0.3	0.1	1.0	0	OL	2.0	27.47	6.5	146.9	D
C37	Titan Type III	0.20	1.0	0.6	0.3	0.1	1.0	0	OL	2.0	27.47	0	149.0	D
C4	Lehigh White Cement	0.20	1.0	0.6	0.3	0.1	1.0	0	OL	2.0	23.35	0	146.5	D







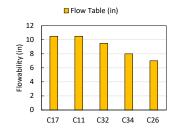
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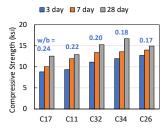
## Experimental Results – Small-Batch

#### Water-to-Binder Ratio

 $\it w/b$  between 0.18 and 0.20 produced highest compressive strength w/ good flow and working time

		w/b		Mix	( Pro	porti	ons		Fil	er	Admix	Density	
Mix.	Cement Type		agg/c m	С	S	SF	FA	UFR	Туре	Content (%)	HRWR (oz./cwt)	VMA (oz./cwt)	(lb/ft³)
C17	Titan Type I/II	0.24	1.0	0.6	0.3	0.1	1.0	0	OL	2.0	16.39	2.47	142.8
C11	Titan Type I/II	0.22	1.0	0.6	0.3	0.1	1.0	0	OL	2.0	19.87	6.5	144.6
C32	Titan Type I/II	0.20	1.0	0.6	0.3	0.1	1.0	0	OL	2.0	27.47	6.5	146.9
C34	Titan Type I/II	0.18	1.0	0.6	0.3	0.1	1.0	0	OL	2.0	27.47	6.5	149.8
C26	Titan Type I/II	0.17	1.0	0.6	0.3	0.1	1.0	0	OL	2.0	35.52	0	150.0





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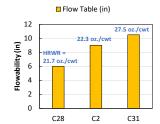
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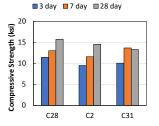
## Experimental Results – Small-Batch

#### **HRWR** Content

Increasing HRWR content decreased compressive strength

			w/b		Mi	k Pro	porti	ons		Fib	er	Admi		
	Mix.	Cement Type		ag/cm	С	S	SF	FA	UFR	Туре	Content (%)	HRWR (oz./cwt)	VMA (oz./cwt)	Density (lb/ft³)
I	C28	Titan Type I/II	0.20	1.0	0.6	0.3	0.1	1	0	OL	2.0	21.70	0	147.1
I	C2	Titan Type I/II	0.20	1.0	0.6	0.3	0.1	1	0	OL	2.0	22.25	0	144.5
	C31	Titan Type I/II	0.20	1.0	0.6	0.3	0.1	1	0	OL	2.0	27.47	0	147.4





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## Experimental Results – Small-Batch

#### Water-to-Binder Ratio

Water-to-cement (w/c):  $W/c = \frac{(W_1 + W_2)}{C_1}$ 

Water-to-binder (w/b):  $w/b = \frac{(W_1 + W_2)}{(C_1 + C_2)}$ 

Modified water-to-binder (w/b):  $w^*/_b = \frac{(W_1 + W_2 + W_3)}{(C_1 + C_2)}$ 

where:

 $W_1$  = weight of free water

 $W_2$  = weight of water available as moisture content in

aggregates

 $W_3$  = weight of liquid portion of chemical admixture

 $C_1$  = weight of cement  $C_2$  = weight of SCMs



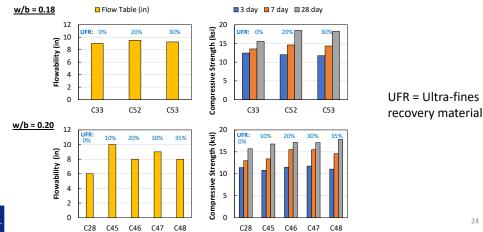
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## Experimental Results – Small-Batch

Fine Aggregate Type and Content

Using UFR at 20% - 30% increased strength, but required more HRWR for flowability



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## Experimental Results - Small-Batch

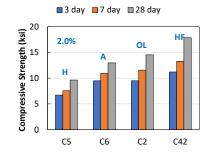
#### **Observed Expansive Behavior**

Uncoated fibers with high zinc contents can lead to an expansive reaction in the UHPC that greatly decreases its strength









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**Note**: The manufacturer of the fiber communicated to the research team that the issue has been fixed, but the testing schedule did not allow for new samples to be cast and tested with the improved fibers



Fiber Type Legend

A = Dramix 4D 65/35BG

OL = Dramix OL 13/.20

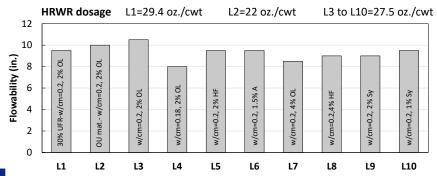
**Sy** = Synthetic Fiber **HF** = Hiper Fiber Type A

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## Experimental Results – Large-Batch

#### Fiber Type and Content – Flowability

(1) Fiber type did not affect flow; (2) Increased fiber content decreased flow; (3) Decreasing w/cm also decreased flow; (4) Use of ultra-fine recovery (UFR) increased HRWR demand



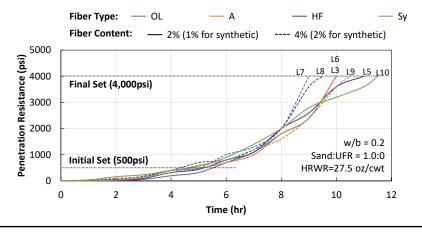


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## Experimental Results – Large-Batch

#### Effect of Fiber Type and Content – Set Time

(1) Increased fiber content decreased set time. (2) Fiber type did not significantly affect set time (similar fiber content had similar set times).



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Fiber Type Legend

A = Dramix 4D 65/35BG

H = Helix 5-13 Uncoated

OL = Dramix OL 13/.20 Sy = Synthetic Fiber HF = Hiper Fiber Type A

Fiber Type Legend

A = Dramix 4D 65/35BG

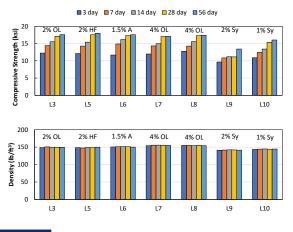
OL = Dramix OL 13/.20

**Sy** = Synthetic Fiber **HF** = Hiper Fiber Type A

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## Experimental Results – Large-Batch

Fiber Type and Content – Compression and Density



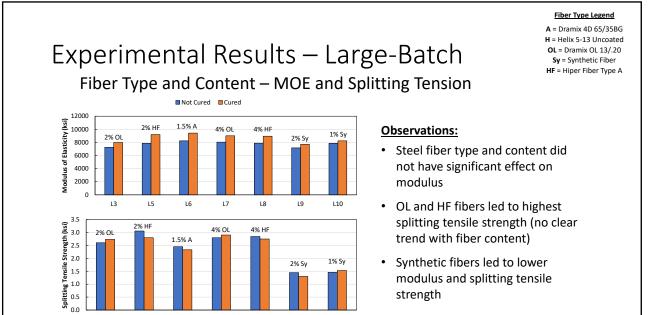
#### **Observations:**

- Similar compressive strength and density for mixtures with steel fibers (regardless of fiber type or content)
- Synthetic fibers led to lower compressive strength and smaller density

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#### Fiber Type Legend A = Dramix 4D 65/35BG H = Helix 5-13 Uncoated Experimental Results - Large-Batch **OL** = Dramix OL 13/.20 Sy = Synthetic Fiber HF = Hiper Fiber Type A Fiber Type and Content - Flexural Strength Modulus of Rupture (ksi) 1.5% A **Observations:** · All steel fibers led to MOR strengths over L9 · Increasing fiber content led to higher Fiber Type: modulus of rupture for OL and HF fibers 2% (1% for synthetic) --- 4% (2% for synthetic) 3.5 · Type A fibers had similar MOR to OL fibers 3.0 (kg) 2.5 Flexural Stress ( HF fibers led to highest MOR strength · Synthetic fibers had lowest MOR strength 0.5 110 0.05 0.10 0.20 0.25

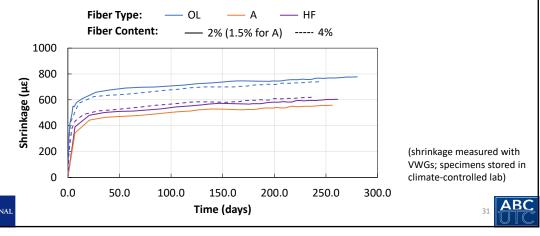
#### Fiber Type Legend

A = Dramix 4D 65/35BG H = Helix 5-13 Uncoated OL = Dramix OL 13/.20 Sy = Synthetic Fiber HF = Hiper Fiber Type A

## Experimental Results – Large-Batch

#### Fiber Type and Content – Shrinkage

HF and A fibers had less shrinkage than OL fiber specimens; similar shrinkage between 2% and 4% with same fiber type



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### Conclusions and Recommendations

- There is a significant amount of research on developing non-proprietary UHPC mixtures using many different types of materials
- 2. You can investigate development of a non-proprietary UHPC mixture using the procedure outlined at the beginning
  - Mix designs can be used in different parts of the country, but it is a good idea to adapt for local materials and at a minimum
    make trial batches
- 3. Some general findings from this work:
  - Fine aggregate moisture had large affect on repeatability of UHPC properties; we oven dried aggregate to solve this issue
  - VMA content did not influence compressive strength and could be used to stabilize heavier steel fibers in the mixtures (about 10 oz./cwt)
  - Use of fibers with 0.5-inch length, 0.008-inch diameter, and tensile strength of 400 ksi led to the best overall performance of the UHPC
  - Uncoated fibers with high zinc contents can lead to expansive reaction in the UHPC that greatly decreases its strength; this
    reaction can be observed in small (0.15 ft<sup>3</sup>) trial batches



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## ABC-UTC Non-Proprietary Mixture

Final Mixture Proportions and Approx. Cost



	Mix.	Cement Type	w/b		Mix P	ropor	tions		Fibe	er	Admixtures		
				ag/cm	С	S	SF	FA	Туре	Content (%)	HRWR (oz./cwt)	VMA (oz./cwt)	
	L3	Titan Type I/II	0.20	1.0	0.6	0.3	0.1	1.00	HF or OL	2.0	27.5	0	

#### **Approximate Cost per Component**

Type I/II Cement: \$100/ton

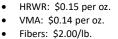
Silica Fume: \$1,000/ton Slag: \$100/ton

Fine Masonry Sand: \$15/ton

UFR: currently not a commercial product



**Approximately** \$800 per cubic yard



Water: \$0.004/gallon (\$0.00048/lb)



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## Thank You

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