



STATE OF CALIFORNIA
DEPARTMENT OF TRANSPORTATION

**NOTICE TO BIDDERS
AND
SPECIAL PROVISIONS**

**FOR CONSTRUCTION ON STATE HIGHWAY IN HUMBOLDT COUNTY NEAR
ARCATA FROM JACOBY CREEK BRIDGE TO GANNON SLOUGH BRIDGE**

In District 01 On Route 101

Under

Bid book dated November 4, 2019

Standard Specifications dated 2018

Project plans approved August 14, 2019

Standard Plans dated 2018

Identified by

Contract No. 01-0E0004

01-Hum-101-84.4/84.8

Project ID 0113000091

SPECIAL NOTICES

- See sections 2 and 3 for contractors' registration requirements.
- See section 2 for submittal requirements for DBE quotes, DVBE quotes, and Non-Small Business Subcontractor Preference.
- For local material from (1) a noncommercial source or (2) a source not regulated under California jurisdiction, you must submit a local material plan and analytical test results for pH, lead, and other constituents for each site. See section 6-1.03B(1) for the specifications.
- See automated machine guidance requirements in sections 5-1.24, 5-1.25, 5-1.26, and 19-1.03A.

CONTRACT NO. 01-0E0004

The special provisions contained herein have been prepared by or under the direction of the following Registered Person:

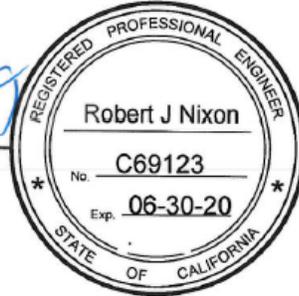
HIGHWAY



8-14-19

REGISTERED CIVIL ENGINEER

DATE



STRUCTURES



Registered Civil Engineer

8/14/2019
Date



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STANDARD PLANS LIST

The standard plan sheets applicable to this Contract include those listed below. The applicable revised standard plans (RSPs) listed below are included in the project plans.

A3A	Abbreviations (Sheet 1 of 3)
A3B	Abbreviations (Sheet 2 of 3)
A3C	Abbreviations (Sheet 3 of 3)
A10A	Legend - Lines and Symbols (Sheet 1 of 5)
A10B	Legend - Lines and Symbols (Sheet 2 of 5)
A10C	Legend - Lines and Symbols (Sheet 3 of 5)
A10D	Legend - Lines and Symbols (Sheet 4 of 5)
A10E	Legend - Lines and Symbols (Sheet 5 of 5)
A10F	Legend - Soil (Sheet 1 of 2)
A10G	Legend - Soil (Sheet 2 of 2)
A10H	Legend - Rock
A20A	Pavement Markers and Traffic Lines - Typical Details
RSP A20B	Pavement Markers and Traffic Lines - Typical Details
A40B	Shoulder Rumble Strip Details - Ground-In Indentations
A40C	Edge Line Rumble Strip Details - Ground-In Indentations
A62C	Limits of Payment for Excavation and Backfill - Bridge
RSP A77L1	Midwest Guardrail System - Standard Railing Section (Wood Post with Wood Block)
A77M1	Midwest Guardrail System - Standard Hardware
A77N1	Midwest Guardrail System - Wood Post and Wood Block Details
RSP A77N3	Midwest Guardrail System - Typical Line Post Embedment and Hinge Point Offset Details
A77U1	Midwest Guardrail System - Connections to Bridge Railings without Sidewalks Details No. 1
A77U2	Midwest Guardrail System - Connections to Bridge Railings without Sidewalks Details No. 2
RSP A77U4	Midwest Guardrail System - Transition Railing (Type WB-31)
P74	Pavement Edge Treatments
P75	Pavement Edge Treatments - Overlays
P76	Pavement Edge Treatments - New Construction
T1A	Temporary Crash Cushion, Sand Filled (Unidirectional)
T1B	Temporary Crash Cushion, Sand Filled (Bidirectional)
T2	Temporary Crash Cushion, Sand Filled (Shoulder Installations)

T3A	Temporary Railing (Type K)
T3B	Temporary Railing (Type K)
T9	Traffic Control System Tables for Lane and Ramp Closures
T10	Traffic Control System for Lane Closure on Freeways and Expressways
T10A	Traffic Control System for Lane Closure on Freeways and Expressways
T14	Traffic Control System for Ramp Closure
RSP T15	Traffic Control System for Moving Lane Closure on Multilane Highways
T56	Temporary Water Pollution Control Details (Temporary Fiber Roll)
T59	Temporary Water Pollution Control Details (Temporary Concrete Washout Facility)
T65	Temporary Water Pollution Control Details (Temporary High-Visibility Fence)
RSP B0-1	Bridge Details
B0-3	Bridge Details
B0-13	Bridge Details
B6-21	Joint Seals (Maximum Movement Rating = 2")
B7-1	Box Girder Details
B9-1	Structure Approach - Type N (30)
B9-5	Structure Approach - Slab Details
B11-75	California ST-70 Bridge Rail (Sheet 1 of 4)
B11-76	California ST-70 Bridge Rail (Sheet 2 of 4)
B11-77	California ST-70 Bridge Rail (Sheet 3 of 4)
B11-78	California ST-70 Bridge Rail (Sheet 4 of 4)
RS1	Roadside Signs - Typical Installation Details No. 1
RS2	Roadside Signs - Wood Post - Typical Installation Details No. 2
RS4	Roadside Signs - Typical Installation Details No. 4

CANCELED STANDARD PLANS LIST

The standard plan sheets listed below are canceled and not applicable to this contract.

Plan No.	Date Canceled	Plan No.	Date Canceled	Plan No.	Date Canceled
C7A	10-19-18				
C7B	10-19-18				
C7C	10-19-18				
B11-55	04-19-19				
B11-56	10-19-18				
B11-57	10-19-18				
ES-2C	10-19-18				

NOTICE TO BIDDERS

Bids open Friday, December 13, 2019

Dated November 4, 2019

General work description: Bridge rail replacement.

The Department will receive sealed bids for CONSTRUCTION ON STATE HIGHWAY IN HUMBOLDT COUNTY NEAR ARCATA FROM JACOBY CREEK BRIDGE TO GANNON SLOUGH BRIDGE.

District-County-Route-Post Mile: 01-Hum-101-84.4/84.8

Contract No. 01-0E0004

The Contractor must have either a Class A license or any combination of the following Class C licenses which constitutes a majority of the work: C-8, C-12.

The Department establishes no DVBE Contract goal but encourages bidders to obtain DVBE participation.

For the Federal training program, the number of trainees or apprentices is 4.

Bids must be on a cost+time basis.

Complete the work within the number of working days bid.

Do not bid less than 155 working days and not more than 220 working days.

The estimated cost of the project is \$8,000,000.

The Department will receive bids until 2:00 p.m. on the bid open date via Bid Express website. Bids received after this time will not be accepted. For more information refer to the Electronic Bidding Guide at the Office Engineer's website.

The Department will open and publicly read the bids through webcast/teleconference services immediately after the specified closing time.

For bid results go to:

<http://ppmoe.dot.ca.gov/des/oe/contractor-info.html>

Select *Electronic Bidding* under the *Bidding* tab.

District office addresses are provided in the *Standard Specifications*.

Present bidders' inquiries to the Department and view the Department's responses at:

<http://ppmoe.dot.ca.gov/des/oe/bid-inquiries.php>

Questions about alleged patent ambiguity of the plans, specifications, or estimate must be asked before bid opening. After bid opening, the Department does not consider these questions as bid protests.

Submit your bid with bidder's security equal to at least 10 percent of the bid.

Under Govt Code § 14835 et seq. and 2 CA Code of Regs § 1896 et seq., the Department gives preference to certified small businesses and non-small businesses who commit to 25 percent certified small business participation.

Under Pub Cont Code § 6107, the Department gives preference to a "California company," as defined, for bid comparison purposes over a nonresident contractor from any state that gives or requires a preference to be given to contractors from that state on its public entity construction contracts.

Prevailing wages are required on this Contract. The Director of the California Department of Industrial Relations determines the general prevailing wage rates. Obtain the wage rates at the DIR website, <http://www.dir.ca.gov>, or from the Department's Labor Compliance Office of the district in which the work is located.

The Department has made available Notices of Suspension and Proposed Debarment from the Federal Highway Administration. For a copy of the notices, go to http://www.dot.ca.gov/hq/esc/oe/contractor_info. Additional information is provided in the Excluded Parties List System at <https://www.epls.gov>.

Caltrans and the Construction Industry are committed to making partnering the way we do business. For more information, go to <http://www.dot.ca.gov/hq/construc/partnering.html>.

Department of Transportation

D01/RJN

BID ITEM LIST

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
1	070030	LEAD COMPLIANCE PLAN	LS	LUMP SUM
2	080050	PROGRESS SCHEDULE (CRITICAL PATH METHOD)	LS	LUMP SUM
3	090105	TIME-RELATED OVERHEAD (LS)	LS	LUMP SUM
4	120090	CONSTRUCTION AREA SIGNS	LS	LUMP SUM
5	120100	TRAFFIC CONTROL SYSTEM	LS	LUMP SUM
6	120110	FLASHING ARROW SIGN	EA	2
7	120120	TYPE III BARRICADE	EA	8
8	120159	TEMPORARY TRAFFIC STRIPE (PAINT)	LF	9,210
9	120165	CHANNELIZER (SURFACE MOUNTED)	EA	160
10	038813	TEMPORARY PORTABLE RADAR FEEDBACK SIGN	EA	2
11	128652	PORTABLE CHANGEABLE MESSAGE SIGN (LS)	LS	LUMP SUM
12	129000	TEMPORARY RAILING (TYPE K)	LF	3,700
13	038814	RELOCATE TEMPORARY RAILING (TYPE K)	LF	3,800
14	038815	ALTERNATIVE TEMPORARY CRASH CUSHION SYSTEM	EA	5
15	038816	RELOCATE ALTERNATIVE TEMPORARY CRASH CUSHION SYSTEM	EA	12
16	129150	TEMPORARY TRAFFIC SCREEN	LF	780
17	130100	JOB SITE MANAGEMENT	LS	LUMP SUM
18	130300	PREPARE STORM WATER POLLUTION PREVENTION PLAN	LS	LUMP SUM
19	130310	RAIN EVENT ACTION PLAN	EA	79
20	130320	STORM WATER SAMPLING AND ANALYSIS DAY	EA	130

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
21	130330	STORM WATER ANNUAL REPORT	EA	3
22	130505	MOVE-IN/MOVE-OUT (TEMPORARY EROSION CONTROL)	EA	2
23	130530	TEMPORARY HYDRAULIC MULCH (BONDED FIBER MATRIX)	SQYD	7,640
24	130640	TEMPORARY FIBER ROLL	LF	2,260
25	130730	STREET SWEEPING	LS	LUMP SUM
26	130900	TEMPORARY CONCRETE WASHOUT	LS	LUMP SUM
27	140003	ASBESTOS COMPLIANCE PLAN	LS	LUMP SUM
28	141120	TREATED WOOD WASTE	LB	20,700
29	146001	CONTRACTOR-SUPPLIED BIOLOGIST (DAY)	DAY	30
30	146003	NATURAL RESOURCE PROTECTION PLAN	LS	LUMP SUM
31	146007	INVASIVE SPECIES CONTROL	LS	LUMP SUM
32	038817	BAT EXCLUSION	LS	LUMP SUM
33	038818	CONTRACTOR SUPPLIED HYDROACOUSTIC MONITOR	LS	LUMP SUM
34	160110	TEMPORARY HIGH-VISIBILITY FENCE	LF	3,450
35	190101	ROADWAY EXCAVATION	CY	2,460
36	190185	SHOULDER BACKING	TONS	260
37(F)	192008	STRUCTURE EXCAVATION (TYPE A)	CY	386
38	192053	STRUCTURE EXCAVATION (TYPE Z-2) (AERIALY DEPOSITED LEAD)	CY	22
39(F)	193003	STRUCTURE BACKFILL (BRIDGE)	CY	93
40	198010	IMPORTED BORROW (CY)	CY	2,770

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
41	202006	SOIL AMENDMENT	CY	0.3
42	202038	PACKET FERTILIZER	EA	24
43	202039	SLOW-RELEASE FERTILIZER	LB	0.8
44	204038	PLANT (GROUP U)	EA	12
45	204099	PLANT ESTABLISHMENT WORK	LS	LUMP SUM
46	205035	WOOD MULCH	CY	0.6
47	038819	FIBER REINFORCED MATRIX	SQFT	41,600
48	260203	CLASS 2 AGGREGATE BASE (CY)	CY	2,260
49	390095	REPLACE ASPHALT CONCRETE SURFACING	CY	7
50	390132	HOT MIX ASPHALT (TYPE A)	TON	1,750
51	390135	HOT MIX ASPHALT (LEVELING)	TON	720
52	390401	HOT MIX ASPHALT-OPEN GRADED (OPEN GRADED FRICTION COURSE)	TON	240
53	038820	HOT MIX ASPHALT-OPEN GRADED (COLORIZED OGFC)	TON	85
54	397005	TACK COAT	TON	2.4
55	398200	COLD PLANE ASPHALT CONCRETE PAVEMENT	SQYD	1,030
56	398300	REMOVE BASE AND SURFACING	CY	10
57	495133	FURNISH 36" CAST-IN-STEEL SHELL CONCRETE PILING	LF	1,816
58	495134	DRIVE 36" CAST-IN-STEEL SHELL CONCRETE PILE	EA	24
59	046919	PRESTRESSING STEEL (TRANSVERSE TIE ROD)	LS	LUMP SUM
60	510000	SEAL COURSE CONCRETE	CY	118

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
61(F)	510053	STRUCTURAL CONCRETE, BRIDGE	CY	231
62(F)	510054	STRUCTURAL CONCRETE, BRIDGE (POLYMER FIBER)	CY	69
63(F)	046920	STRUCTURAL CONCRETE, BRIDGE (RSC)	CY	93
64	046921	STRUCTURAL CONCRETE, BRIDGE (RSC) PATCH	CF	87
65(F)	046922	STRUCTURAL CONCRETE, BARRIER SLAB (RSC)	CY	75
66(F)	510088	STRUCTURAL CONCRETE, APPROACH SLAB (TYPE N MODIFIED)	CY	113
67	511106	DRILL AND BOND DOWEL	LF	1,404
68	512224	FURNISH PRECAST PRESTRESSED CONCRETE BOX GIRDER (70'-80')	EA	9
69	512502	ERECT PRECAST PRESTRESSED CONCRETE BOX GIRDER	EA	9
70	519081	JOINT SEAL (MR 1/2")	LF	167
71	519091	JOINT SEAL (MR 1 1/2")	LF	88
72(F)	520102	BAR REINFORCING STEEL (BRIDGE)	LB	77,100
73(F)	520110	BAR REINFORCING STEEL (EPOXY COATED) (BRIDGE)	LB	92,610
74(F)	550102	STRUCTURAL STEEL (BRIDGE)	LB	608
75	600029	REMOVE ASPHALT CONCRETE SURFACING	SQFT	6,883
76	600033	REMOVE UNSOUND CONCRETE	CF	87
77	600037	PREPARE CONCRETE BRIDGE DECK SURFACE	SQFT	6,883
78	600097	BRIDGE REMOVAL	LS	LUMP SUM
79	046923	BRIDGE SUPERSTRUCTURE MOVE	LS	LUMP SUM
80	600115	BRIDGE REMOVAL (PORTION), LOCATION A	LS	LUMP SUM

Item No.	Item Code	Item Description	Unit of Measure	Estimated Quantity
81	600116	BRIDGE REMOVAL (PORTION), LOCATION B	LS	LUMP SUM
82	046924	PLUG DECK DRAIN	EA	3
83	782200	OBLITERATE SURFACING	SQYD	2,730
84	810120	REMOVE PAVEMENT MARKER	EA	84
85	810230	PAVEMENT MARKER (RETROREFLECTIVE)	EA	120
86	820110	MILEPOST MARKER	EA	2
87	820134	OBJECT MARKER (TYPE P)	EA	6
88	832005	MIDWEST GUARDRAIL SYSTEM	LF	1,000
89	839543	TRANSITION RAILING (TYPE WB-31)	EA	7
90	839584	ALTERNATIVE IN-LINE TERMINAL SYSTEM	EA	6
91(F)	046925	CALIFORNIA ST-70 BRIDGE RAIL	LF	582
92	839752	REMOVE GUARDRAIL	LF	800
93	840502	THERMOPLASTIC TRAFFIC STRIPE (ENHANCED WET NIGHT VISIBILITY)	LF	5,990
94	846020	REMOVE PAINTED TRAFFIC STRIPE	LF	6,450
95	846030	REMOVE THERMOPLASTIC TRAFFIC STRIPE	LF	4,150
96	846046	6" RUMBLE STRIP (ASPHALT CONCRETE PAVEMENT)	STA	17
97	846051	12" RUMBLE STRIP (ASPHALT CONCRETE PAVEMENT)	STA	17
98	046926	4" CONDUIT (BRIDGE)	LS	LUMP SUM
99	999990	MOBILIZATION	LS	LUMP SUM

AA

2 BIDDING

Add between the 1st and 2nd paragraphs of section 2-1.06B:

The Department makes the following supplemental project information available:

Supplemental Project Information

Means	Description
Included in the <i>Information Handout</i>	1. PLAC - CA-FWS - 1600-2018-0503-R1 Permit 2. PLAC - USFWS - Biological Opinion 3. PLAC - Army Corp of Engineers - Permit 4. PLAC - 1-18-1078-Coastal_Development_Permit 5. PLAC - North Coast Regional Water Quality Control Board WDID No. 1B190035WNHU 6. PLAC - Humboldt Bay Harbor, Recreation and Conservation District Permit 7. PLAC - CA-FWS 1600-2018-0503-R1 Application 8. PLAC - Army Corp of Engineers -404-Application 9. Jacoby Creek Bridge Final Hydraulic Report (04-0313L) 10. Revised Foundation Report for Jacoby Creek Bridge (Left) 11. MTCB Portable Vehicle Washer 12. 04-0023 Jacoby Creek ACM LCP 2013 13. 04-0024 Gannon Slough ACM LCP 2013 14. Aerially Deposited Lead Report 15. Aerially Deposited Lead Report Supplemental
Available as specified in the <i>Standard Specifications</i>	1. 01-0E000-XS-DET-Line 2. 01-0E000-XS-SB1-Line
Included with the project plans	1. Logs of test borings
Available for inspection at the Transportation Laboratory	Rock cores RC-18-0001, RC-18-002

AA

5 CONTROL OF WORK

Add to the end of section 5-1.20A:

During the progress of the work under this Contract, work under the following contracts may be in progress at or near the job site of this Contract:

Coincident or Adjacent Contracts

Contract no.	County–Route–Post Mile	Location	Type of work
01-0C9304	HUM-101-80.8/85.0	Near Arcata	Improve Drainage
01-0C9704	HUM-101-79.9/86.3	In/Near Arcata	Upgrade Guardrail
01-0F2204	HUM-101-80.2/85.8	In/Near Arcata	Upgrade Accel/Decel Lanes
01-366004	HUM-101-80.6/84.0	Near Arcata	Upgrade 4-Lane Facility

Add to the RSS for section 5-1.24:

5-1.24B Department Construction Surveys for Automated Machine Guidance

The Department sets control points to a minimum of 0.07 foot local horizontal accuracy and 3rd order vertical accuracy standards.

For slope stakes and rough grade stakes, the Department sets 6 survey control points or 2 per mile, whichever is greater.

The Department sets slope stakes and rough grade stakes at:

1. Conform stations
2. Beginning and end of each alignment
3. Midpoint or every 200 feet, whichever results in a greater number of stakes, on a curve
4. Every 500 feet on tangents

The Department sets final grade stakes under Chapter 12, "Construction Surveys," section 12.5-6 of the Department's *Surveys Manual*.

At your request, the Department sets survey control points under section 12.1-6, "Automated Machine Guidance." When control stakes are requested, final grade stakes are set at:

1. Conform stations
2. Beginning and end of each alignment
3. Midpoint or every 100 feet, whichever results in the greater number of stakes, on a curve with a radius of 1,200 feet or less
4. Midpoint or every 200 feet, whichever results in the greater number of stakes, on a curve with a radius of more than 1,200 feet
5. Every 200 feet on a tangent

At your request and under Chapter 12 of the Department's *Surveys Manual*, the Department provides (1) staking for intersections, clearing, fencing, drainage, curbs, structures, abutment fill, wall, and miscellaneous areas and (2) additional survey control or staking for earthwork in areas where global navigation satellite system coverage is inadequate for automated machine guidance.

Replace section 5-1.25 with:

5-1.25 AUTOMATED MACHINE GUIDANCE

5-1.25A General

Use automated machine guidance (AMG) for earthwork. AMG must meet or exceed the staking tolerances described in section 12.5, "Typical Department-Furnished Construction Stakes," of the Department's *Surveys Manual*.

Furnish a GNSS rover that functions using the Department's Real Time Network (RTN). The Department returns the GNSS rover upon work completion. This is change order work.

At the preconstruction conference, be prepared to discuss survey control points, site and equipment calibration, inspection methods, conflict resolution, and staking.

5-1.25B Definitions

automated machine guidance (AMG): Technology that uses positioning devices, singly or in combination, such as global navigation satellite systems (GNSS), total stations, or rotating laser levels, to determine and control the real-time position of construction equipment using onboard computer equipment.

California Coordinate System of 1983 (CCS83): CCS83 as defined in Pub Res Code § 8801.

digital construction model (DCM): Three-dimensional model used by the Contractor's AMG equipment.

digital design model: Three-dimensional model consisting of roadway design alignments, profiles, and cross sections representing the finished grade.

digital terrain model: Three-dimensional model representing the original ground before job site activities start.

global navigation satellite system (GNSS): Satellite system used to pinpoint the geographic location of a user's receiver anywhere in the world. Two GNSS systems are in operation: the US GPS and the Russian Federation's GLONASS. Each of the GNSS systems uses a constellation of orbiting satellites working in conjunction with a network of ground stations.

GNSS base station: Single ground-based system consisting of a GNSS receiver, antenna, and telemetry equipment that provides differential GNSS correction signals to other GNSS receivers or rovers. Multiple base stations can be combined into a GNSS network.

GNSS correction service subscription: Subscription service to receive differential GNSS correction signals for higher accuracy GNSS positioning without the need of a GNSS base station. Signals are normally received via cellular wireless data services.

GNSS rover: Portable GNSS antenna, receiver, rod, and data collector with telemetry equipment for real-time point measurements.

grid: Cartesian coordinate system of Northing (y) and Easting (x) coordinates using CCS83.

robotic total station: Survey instrument capable of tracking an optical target and providing real-time coordinates of the target to the equipment operator and AMG equipment. A robotic total station unit can provide AMG if site conditions do not allow GNSS receivers to be used and if a higher accuracy is required than the GNSS provides.

site calibration or localization: Process that establishes the relationship between the observed control point coordinates and the site coordinate system, which is usually grid. The term applies to both GNSS and robotic total station equipment.

5-1.25C Electronic Files

Electronic design files include:

1. Digital terrain model in 3-D DGN or LandXML format
2. Roadway design alignments and profiles in LandXML format
3. Cross sections in 2-D DGN and PDF
4. Digital design model in LandXML format
5. Target lines used to design the corridor in DWG format

The Department makes electronic design files available as supplemental project information.

You must create the digital construction models.

Convert the electronic design files to a format compatible with your AMG system. Manipulation of the electronic design files is at your own risk.

Submit copies of the digital construction model files and any updates to them in LandXML format.

Digital design model information may not exist for contour grading and some drainage areas. The Department places stakes for these areas.

The Department provides you with updated electronic data whenever the Engineer determines a plan change materially affects the finished grade. For minor grade changes, the Department places stakes and marks.

5-1.25D Quality Control Plan

Submit an AMG QC plan at least 15 days before starting work requiring AMG. The plan must include the following information:

1. Contract number
2. Name and contact information of the AMG QC technician

3. Limits of the area for which the AMG will be used
4. Scope of work to be completed using AMG for the following work categories:
 - 4.1. Clearing and grubbing
 - 4.2. Earthwork
 - 4.3. Trench excavation
 - 4.4. Rough grading
 - 4.5. Subgrade
 - 4.6. Subbase
 - 4.7. Base
 - 4.8. Curb and gutter
 - 4.9. Cold planning or milling existing pavement
 - 4.10. Paving
 - 4.11. Intelligent compaction
 - 4.12. Concrete barrier
 - 4.13. Finishing roadway
5. Project control plan sheet detailing control points covering the job site
6. List of GNSS equipment, including:
 - 6.1. Type
 - 6.2. Manufacturer
 - 6.3. Model
 - 6.4. Software version
7. Description of GNSS site calibration or localization checking, including:
 - 7.1. List of equipment requiring calibration or localization checking
 - 7.2. Site calibration or localization procedures
 - 7.3. Frequency of calibration or localization
 - 7.4. Format for recording calibrations or localizations, including:
 - 7.4.1. Date
 - 7.4.2. Locations where calibration or localization was performed
 - 7.4.3. GNSS equipment manufacturer and model
 - 7.4.4. Range of required tolerance
 - 7.4.5. Name and signature of the person performing calibration or localization
 - 7.5. Reporting time for submitting records of calibration or localization
8. Description of daily GNSS equipment or robotic total station equipment check-testing procedures, including the format for recording daily check testing
9. List of AMG onboard computer equipment, including:
 - 9.1. Type
 - 9.2. Manufacturer
 - 9.3. Software version
 - 9.4. List of AMG-controlled equipment, including:
 - 9.4.1. Type, such as loader or grader
 - 9.4.2. Manufacturer
 - 9.4.3. Model
10. Procedures for AMG-controlled equipment calibration, including:
 - 10.1. Description of equipment calibration procedures
 - 10.2. Frequency of calibration
 - 10.3. Format for recording calibration information
11. Electronic data file control, including:
 - 11.1. Name and contact information of the person responsible for the electronic files
 - 11.2. DCM file-naming convention
 - 11.3. Description of the process that will be used to upload the DCM to the AMG equipment
 - 11.4. Description of the process that will be used whenever updated DCM files are required to be uploaded to the AMG equipment

If QC procedures or personnel change, submit a QC plan supplement describing the change.

5-1.25E Quality Control Technician

During AMG activities, provide a QC technician to be responsible for:

1. GNSS site calibration or localization and upload to all GNSS receivers

2. Maintenance of GNSS and AMG equipment
3. Documentation of the calibration or localization and maintenance of GNSS equipment
4. Daily calibration and documentation of AMG equipment
5. Daily setup and takedown of GNSS and robotic total station components

5-1.25F Just-in-Time Training

Provide at least 8 hours of JIT training on the GNSS rover for up to 3 Department employees. Provide training materials and equipment.

The JIT training must cover the following topics:

1. Background information for the GNSS to be used
2. Setup and calibration checks for:
 - 2.1. GNSS receiver
 - 2.2. GNSS base station
 - 2.3. GNSS rovers
 - 2.4. Machinery
3. Operation of the GNSS rover, including:
 - 3.1. Setup data collection
 - 3.2. Settings for alignments and profiles
 - 3.3. Onboard display options
4. Demonstration of grade checking using the rover

The training is change order work.

5-1.25G Construction

5-1.25G(1) General

If you find a discrepancy in any survey control point, survey stake, or in the electronic data provided, immediately, submit an RFI.

5-1.25G(2) GNSS Site Calibration or Localization

Perform GNSS site calibration or localization to the survey control points at least 5 business days before starting work requiring AMG.

Check each survey control point for accuracy. Submit the GNSS site calibration or localization results within 1 business day of the calibration or localization testing. Allow 3 business days for the review of the results

5-1.25G(3) GNSS Check Testing

Before starting daily work requiring AMG, conduct check testing for the proper setup of the GNSS or robotic total station equipment. Ensure the GNSS or robotic total station equipment achieves accuracies within:

1. 0.10 foot in both horizontal and vertical directions for rough grading
2. 0.05 foot in horizontal directions and 0.02 foot in vertical directions for final grades

Before starting daily production, conduct check testing of the AMG equipment and the GNSS rovers.

Within 1 business day after check testing, submit the check-testing results as informational submittals.

5-1.25G(4) Grade Verification

If requested, provide a GNSS rover and personnel to operate it for the Engineer's use in verifying grades. This is change order work.

Replace section 5-1.26 with:

5-1.26 GRADE QUALITY CONTROL

Use a GNSS rover, robotic total station equipment, or a level to check the grades at the frequencies shown in the following table:

Grade Checking Requirements

Type of work	Area or distance represented by the grade checking	Frequency (number of grade points)
Earthwork for cut and fill slopes ≤ 15 feet	200 feet	2
Earthwork for cut and fill slopes > 15 feet	1,000 sq yd	1
Rough grading	1,000 sq yd	1
Trenching	100 feet	6
Subgrade	1 mi	30
Subbase layer	1 mi	50
Base layer	1 mi	100
Curb and gutter	100 feet	6
Concrete barrier	100 feet	5
Finishing roadway	1,000 sq yd	2

Increase the frequency of grade checking of a roadway:

1. Wherever its curve radius is 500 feet or less
2. In areas of a superelevation transition
3. At intersections

Notify the Engineer when an area is ready for line and grade inspection. Submit the grade checking results on a Grade Checking Report form as an informational submittal.

Add between the 2nd and 3rd paragraphs of section 5-1.32:

Where State-owned areas have been designated for Contractor's use beneath bridge structures, comply with the following:

1. Do not store any of the following beneath structures:
 - 1.1 Explosives or explosive materials
 - 1.2 Flammable or combustible materials
 - 1.3 Incompatible materials, such as chlorine and ammonia, or batteries and fuels, in the same secondary containment facility
2. Material storage may not encroach on any of the following:
 - 2.1 Within 20 feet of any bridge support
 - 2.2 Within 10 feet of any exposed footing or pile cap
 - 2.3 Within a 6-foot minimum clear zone height from the bottom of superstructure to top of material storage
3. Maintain 12-foot minimum width pathways beneath each hinge, bent cap and bridge span allowing manlift vehicle access
4. Do not obstruct drainage systems

Add to the end of section 5-1.32:

Personal vehicles of your employees must not be parked on the traveled way or shoulders, including sections closed to traffic.

The plan must be sealed and signed by an engineer who is registered as a civil engineer in the State or a professional geologist licensed as a professional geologist by the State.

If the plan requires revisions, the Engineer provides comments. Submit a revised plan within 7 days of receiving comments. Allow 7 days for the review.

6-1.03B(2) Analytical Test Results

At least 15 days before placing local material, submit analytical test results for each local material obtained from a noncommercial source or a source not regulated under CA jurisdiction. The analytical test results must include:

1. Certification signed by an engineer who is registered as a civil engineer in the State or a professional geologist licensed as a professional geologist by the State stating:

The analytical testing described in the local material plan has been performed. I performed a statistical analysis of the test results using the US EPA's ProUCL software with the applicable 95 percent upper confidence limit. I certify that the material from the local material source is suitable for unrestricted use at the job site, it has a pH above 5.0, does not contain soluble lead in concentrations equal to or greater than 5mg/l as determined by the Waste Extraction Test (WET) Procedures, 22 CA Code of Regs § 66261.24(a)(2) App II, does not contain lead in concentrations above 80 mg/kg total lead, is free from all other contaminants identified in the local material plan, and will comply with the job site's basin plan and water quality objectives of the RWQCB.

2. Chain of custody of samples
3. Analytical results no older than 1 year
4. Statistical analysis of the data using US EPA's ProUCL software with a 95 percent upper confidence limit
5. Comparison of sample results to hazardous waste concentration thresholds and the RWQCB's basin plan requirements and water quality objectives for the job site location

6-1.03B(3) Sample and Analysis

Sample and analyze local material from a (1) noncommercial source or (2) source not regulated under CA jurisdiction:

1. Before bringing the local material to the job site
2. As described in the local material plan
3. Under US EPA Test Methods for Evaluating Solid Waste, Physical/Chemical Methods (SW-846)

The sample collection must be designed to generate a data set representative of the entire volume of proposed local material.

Before excavating at the (1) noncommercial material source or (2) a source not regulated under CA jurisdiction, collect the minimum number of samples and perform the minimum number of analytical tests for the corresponding maximum volume of local material as shown in the following table:

Minimum Number of Samples and Analytical Tests for Local Material

Maximum volume of imported borrow (cu yd)	Minimum number of samples and analytical tests
< 5,000	8
5,000–10,000	12 for the first 5,000 cu yd plus 1 for each additional 1,000 cu yd or portion thereof
10,000–20,000	17 for the first 10,000 cu yd plus 1 for each additional 2,500 cu yd or portion thereof
20,000–40,000	21 for the first 20,000 cu yd plus 1 for each additional 5,000 cu yd or portion thereof
40,000–80,000	25 for the first 40,000 cu yd plus 1 for each additional 10,000 cu yd or portion thereof
> 80,000	29 for the first 80,000 cu yd plus 1 for each additional 20,000 cu yd or portion thereof

Do not collect composite samples or mix individual samples to form a composite sample.

Analyze the samples using the US EPA's ProUCL software with a 95 percent upper confidence limit. All chemical analysis must be performed by a laboratory certified by the SWRCB's Environmental Laboratory Accreditation Program (ELAP).

The analytical test results must demonstrate that the local material:

1. Is not a hazardous waste
2. Has a pH above 5.0
3. Has an average total lead concentration, based upon the 95 percent upper confidence limit, at or below 80 mg/kg
4. Is free of possible contaminants identified in the local material plan
5. Complies with the RWQCB's basin plan for the job site location
6. Complies with the RWQCB's water quality objectives for the job site location

6-1.03C Local Material Management

Do not place local material until authorized.

If the Engineer determines the appearance, odor, or texture of any delivered local material suggests possible contamination, sample and analyze the material. The sampling and analysis is change order work unless (1) hazardous waste is discovered or (2) the analytical test results indicate the material does not comply with section 6-1.03B(3).

Dispose of noncompliant local material at an appropriately permitted CA Class I, CA Class II or CA Class III facility. You are the generator of noncompliant local material.

Add to section 6-1:

6-1.06 BUY CLEAN CALIFORNIA ACT

6-1.06A General

The following materials or products are subject to the Buy Clean California Act (Pub Cont Code § 3500 et seq.):

Material or product	Material specifications
Carbon steel rebar	Section 52-1.02B, "Bar Reinforcement"
Structural steel	Section 55-1.02D(1), "General," – Structural Steel table or Section 99, "Building Construction"
Flat glass	Section 99, "Building Construction"
Mineral wool board insulation	Section 99, "Building Construction"

The sign must be a wood-post sign complying with section 82-3.

The sign panels must be framed, single-sheet aluminum panels complying with section 82-2.

The background on the sign must be Type XI retroreflective sheeting. The Type XI retroreflective sheeting must be on the Authorized Material List for signing and delineation materials.

The legend must be retroreflective except for nonreflective black letters and numerals. The blue and fluorescent orange must match the color specifications available at the FHWA's MUTCD website.

The legend for the type of project must read as follows:

BRIDGE CONSTRUCTION

The legend for the types of funding on a construction project funding sign must read as follows and in the following order:

FEDERAL HIGHWAY TRUST FUNDS

STATE HIGHWAY FUNDS

The Engineer provides the year of completion for the legend on the sign. Install a sign overlay for the year of completion within 15 days of notification.

Do not add information to the construction project funding sign unless authorized.

Replace *Reserved* in section 12-3.11C(3) with:

Install two 96 by 60 inch construction project funding identification signs at the locations determined by the Engineer before starting major work activities visible to highway users.

Replace section 12-3.24 with:

12-3.24 ALTERNATIVE TEMPORARY CRASH CUSHION

12-3.24A General

12-3.24A(1) Summary

Section 12-3.24 includes specifications for constructing alternative temporary crash cushion.

Alternative temporary crash cushion includes everything needed to attach it to the barrier, wall, or other system as shown and as approved by the manufacturer.

12-3.24A(2) Definitions

Not Used

12-3.24A(3) Submittals

Submit a certificate of compliance for each alternative temporary crash cushion used.

At least 10 days before installation, submit a minimum of two copies of the manufacturer's drawings, installation instruction manual, and maintenance manual for each model of alternative temporary crash cushion to be used.

12-3.24A(4) Quality Control

You must have a copy of the manufacturer's drawings, installation instructions manual, and maintenance manual for each alternative temporary crash cushion to be used on the job site during installation.

12-3.24B Materials

Allowable alternative temporary crash cushion systems are one of the following or a Department-authorized equal and must meet Test Level 3 criteria:

1. CRASH CUSHION (TYPE ABSORB) – Crash cushion (Type Absorb) must be an ABSORB 350, 9-element system, manufactured by Barrier Systems, Inc. The ABSORB 350, 9 element system can be obtained from the distributor:

Address	Telephone no.
STATEWIDE SAFETY AND SIGNS, INC. 130 GROBRIC COURT FAIRFIELD, CA 94533	(800) 770-2644 (800) 559-7080

2. CRASH CUSHION (TYPE ACZ 350) – Crash cushion (Type ACZ 350) must be an ACZ 350, manufactured by Energy Absorption Systems, Inc.. The ACZ 350, system can be obtained from the distributors:

Address	Telephone and fax nos.
TRAFFIC CONTROL SERVICE, INC. 8585 THYS COURT SACRAMENTO CA 95828	Telephone: (916) 387-9733 Fax: (916) 387-9734
NATIONAL TRENCH SAFETY 41655 OSGOOD ROAD FREMONT, CA. 94539	Telephone: (510) 490-2140

3. Type SLED-SENTRY LONGITUDINAL ENERGY DISSIPATOR END TREATMENT A three module gating, non-redirective alternative temporary crash cushion must be SLED Alternative Temporary Crash Cushion manufactured by Traffix Devices, Inc., and must include the connection components. Type SLED alternative temporary crash cushion - Type SLED alternative temporary crash cushion must be test level 3, manufactured by Traffix Devices, Inc.,. The SLED alternative temporary crash cushion can be obtained from the manufacturer:

Address	Telephone no.
TRAFFIX DEVICES, INC. 160 AVENIDA LA PATA SAN CLEMENTE, CA 92673	(949) 361-5663 (949) 350-7048 FAX: (949) 361-9205

12-3.24C Construction

Use personnel trained by the manufacturer to install the alternative temporary crash cushion.

Install and maintain the alternative temporary crash cushion under the manufacturer's instructions and as described.

The alternative temporary crash cushion must not encroach on the traveled way.

Secure the alternative temporary crash cushion in place before starting an activity requiring an alternative temporary crash cushion.

Maintain the alternative temporary crash cushion in place at each location, including times when work is not actively in progress. You may remove an alternative temporary crash cushion during the work shift for access to the work if the exposed fixed obstacle is 15 feet or more from the nearest lane carrying traffic. Reset the alternative temporary crash cushion before the end of the work shift.

Repair damaged alternative temporary crash cushion immediately. Remove and replace alternative temporary crash cushions damaged beyond repair.

Attach a Type R or Type P marker panel to the front of the alternative temporary crash cushion if the closest point of the alternative temporary crash cushion is within 12 feet of the traveled way. Firmly fasten the marker panel to the alternative temporary crash cushion with commercial quality hardware or by other authorized methods.

Remove temporary alternative crash cushion, including marker panels, when no longer required for the work. Do not install an alternative temporary crash cushion in the permanent work.

12-3.24D Payment

Relocating or resetting alternative temporary crash cushion is change order work if ordered and not shown.

Replacement and repair of alternative temporary crash cushions damaged by public traffic is change order work.

Add to the beginning of section 12-3.32C:

Place PCMSs at the locations shown and in advance of the 1st warning sign for each:

1. Stationary lane closure
2. Shoulder closure

Add between the 9th and 10th paragraphs of section 12-3.32C:

Start displaying the message on the sign 15 minutes before closing the lane or shoulder or when directed by the Engineer.

Replace section 12-3.37 with:

12-3.37 PORTABLE VEHICLE SPEED FEEDBACK SIGNS

12-3.37A General

12-3.37A(1) Summary

Section 12-3.37 includes specifications for placing portable vehicle speed feedback signs.

12-3.37A(2) Definitions

Not Used

12-3.37A(3) Submittals

If requested, submit a certificate of compliance for each portable vehicle speed feedback sign.

12-3.37A(4) Quality Assurance

Each portable vehicle speed feedback sign must comply with section 87-14.01D(2) except testing must be performed off-site prior to delivery to the project.

12-3.37B Materials

Each portable vehicle speed feedback sign must comply with section 87-14.02 .

Provide the following material and equipment mounted on a trailer:

1. Power supply with backup
2. Sign panel (R2-1)

The portable vehicle speed feedback sign must be a self-contained unit that can be delivered to the job site and placed into immediate operation. The sign unit must be unaffected by unauthorized mobile-radio transmissions.

The trailer must be equipped so that it can be leveled and plumbed.

A minimum of 3 feet of retroreflective material must be permanently affixed on all 4 sides of the trailer. The retroreflective material need not be continuous but must be visible on the same plane.

The LED character display must remain blank when the detected vehicle speed is 10 miles or less than the pre-set speed.

12-3.37C Construction

Operate the portable vehicle speed feedback sign under the manufacturer’s instructions.

Place sign as far from the traveled way as practicable where it is legible to approaching traffic without encroaching on the traveled way. Where the vertical roadway curvature restricts the sight distance of approaching traffic, place the sign on or before the crest of the curvature where it is most visible to the approaching traffic. Where the horizontal roadway curvature restricts the sight distance of approaching traffic, place the sign at or before the curve where it is most visible to approaching traffic. Where practicable, place the sign behind guardrail or Type K temporary railing.

When placed outside of a lane closure, make a taper consisting of 9 traffic cones placed 25 feet apart to delineate the location of a sign except where the sign is placed behind guardrail or Type K temporary railing.

When placed within a lane closure, place the sign after the buffer zone and in advance of the work area.

Keep the sign clean to provide maximum visibility.

Configure the portable vehicle speed feedback sign system to detect only traffic in the approach direction of travel.

Operate the sign under the manufacturer’s instructions

After initial placement, move the sign from location to location as required.

When a sign is not in use, move the sign to an area at least 15 feet from the edge of the traveled way or remove it from the job site away from traffic.

12-3.37D Payment

Not Used

Add to section 12-4.02A(2):

special days: Special days are shown in the following table:

Special Days		
Event	Event date	Special days
Humboldt State University Graduation	Second weekend in May	Friday through Sunday
Kinetic Grand Championship Race	Last Weekend in May	Friday through Monday
Humboldt Bay Marathon	Second Sunday in August	Sunday

Verify the dates prior to beginning work.

Add between the 1st and 2nd paragraphs of section 12-4.02A(3)(c):

Submit a contingency plan for each of the following activities:

1. Bridge removal
2. Bridge superstructure move
3. Structural concrete, bridge (RSC)
4. Concrete barrier slab
5. Asphalt grinding

Add to the end of section 12-4.02C(1):

Keep the full width of the traveled way open to traffic when no active construction activities are occurring in the traveled way or within 6 feet of the traveled way.

Keep the full width of the ramp traveled way open for use by traffic on designated holidays and special days.

For each 10-minute interval or fraction thereof past the time specified to open the closure, the amount for liquidated damages per interval shown in the table below is deducted. Liquidated damages are limited to 5 percent of the total bid per occurrence. Liquidated damages are not assessed if the Engineer orders the closure to remain in place beyond the scheduled pickup time.

Type of facility	Route	Direction or segment	Period	Liquidated damages/interval
Mainline	101	NB/SB	1st half hour	\$1,000/10 minutes
			2nd half hour	\$1,000/10 minutes
			2nd hour and beyond	\$1,000/10 minutes

Add to the end of section 12-4.02C(3)(a):

If you use an impact attenuator vehicle as a shadow vehicle, you are not required to close the adjacent traffic lane for the following activities:

1. Grinding

If work vehicles or equipment are parked on the shoulder within 6 feet of a traffic lane of a freeway or expressway, close the shoulder area as shown.

Replace *Reserved* in section 12-4.02C(3)(f) with:

Closure restrictions for designated holidays and special days are shown in the following table:

Lane Closure Restrictions For Designated Holidays And Special Days											
Thu	Fri	Sat	Sun	Mon	Tues	Wed	Thu	Fri	Sat	Sun	Mon
xx	H xx										
	SD xx										
	xx	H xx									
		SD xx									
	xx		H xx	xx							
			SD xx								
	xx			H xx							
				SD xx							
				xx	H xx						
					xx	H xx					
						xx	H xx				
							xx	H xx	xx		
Legend:											
	Refer to lane requirement charts.										
xx	At least two adjacent through lanes in each direction of travel must be open for use by traffic.										
H	Designated holiday										
SD	Special day										

Replace Reserved in section 12-4.02C(3)(g) with:

Freeway/Expressway lane closures must comply with the requirements shown in the following charts:

Chart No. G1																									
Freeway/Expressway Lane Requirements																									
County: Humboldt							Route/Direction: 101 NB/SB							Post Mile: 84.4/84.8											
Closure limits: At the NB & SB Jacoby Creek Bridges (#04-0023) and NB Gannon Slough Bridge (#04-0024R)																									
Hour	00	01	02	03	04	05	06	07	08	09	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon– Thu	1	1	1	1	1	1																1	1	1	1
Fri	1	1	1	1	1	1																			
Sat																									
Sun																						1	1	1	1
Legend:																									
<input type="checkbox"/> 1 Provide at least one 16 foot through freeway/expressway lane open in direction of travel. The maximum closure length is 4,000 feet.																									
<input type="checkbox"/> No lane and/or shoulder closures allowed.																									
REMARKS:																									

Chart No. G2																									
Freeway/Expressway Lane Requirements																									
County: Humboldt							Route/Direction: 101 NB							Post Mile: 84.4/84.8											
Closure limits: At the NB Jacoby Creek Bridge (#04-0023R) and NB Gannon Slough Bridge (#04-0024R)																									
Hour	00	01	02	03	04	05	06	07	08	09	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon	1	1	1	1	1	1																			
Tue– Thu																									
Fri																						1	1	1	1
Sat	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
Sun	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1	1
Legend:																									
<input type="checkbox"/> 1 Provide at least one 16 foot through freeway/expressway lane open in direction of travel. The maximum closure length is 4,000 feet.																									
<input type="checkbox"/> No lane and/or shoulder closures allowed.																									
REMARKS:																									
1. This chart to be used when the existing AC bridge deck surface is removed and replaced with a reinforced concrete bridge deck. 2. This chart is valid for 4 occurrences only.																									

Replace Reserved in section 12-4.02C(3)(h) with:

Comply with the requirements for the complete Freeway/Expressway closure shown in the following charts:

Chart No. H1																									
Complete Freeway/Expressway Closure Hours																									
County: Humboldt							Route/Direction: SB 101							Post Mile:83.9/86.2											
Closure limits:																									
Hour	00	01	02	03	04	05	06	07	08	09	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon–Thu																									
Fri																									C
Sat	C	C	C	C	C																				
Sun																									
Legend:																									
<input type="checkbox"/> C Freeway/Expressway may be closed completely.																									
<input type="checkbox"/> No complete closure is allowed.																									
REMARKS:																									
1. SB Route 101 traffic will be detoured to SB Route 255 for access to Eureka. 2. This chart is valid during the one, 6-hr SB 101 complete closure for the Jacoby Creek Bridge (#04-0023L) trial bridge slide. It is anticipated that this closure will happen 1 week prior to the final SB 101 Jacoby Creek Bridge (#04-0023L) slide.																									

Chart No. H2																									
Complete Freeway/Expressway Closure Hours																									
County: Humboldt							Route/Direction: SB 101							Post Mile:83.9/86.2											
Closure limits:																									
Hour	00	01	02	03	04	05	06	07	08	09	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon–Thu																									
Fri																						C	C	C	C
Sat	C	C	C	C	C	C																			
Sun																									
Legend:																									
<input type="checkbox"/> C Freeway/Expressway may be closed completely.																									
<input type="checkbox"/> No complete closure is allowed.																									
REMARKS:																									
1. SB Route 101 traffic will be detoured to SB Route 255 for access to Eureka. 2. This chart is valid for one, 10-hr closure, when the new SB 101 Jacoby Creek Bridge (#04-0023L) is being moved from its temporary to final alignment.																									

Replace *Reserved* in section 12-4.02C(3)(j) with:

Comply with the requirements for the complete ramp closure shown in the following chart:

Chart No. J1 Complete Ramp Closure Hours																									
County: Humboldt							Route/Direction: 101 SB							Post Mile: 84.975, 85.706 & 85.707											
Closure limits: SB Route 101 onramps from G Street, NB Route 255 and SB Route 255																									
Hour	00	01	02	03	04	05	06	07	08	09	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24
Mon–Thu																									
Fri																						C	C	C	C
Sat	C	C	C	C	C	C																			
Sun																									
Legend:																									
C	Ramp may be closed completely.																								
	No ramp closures allowed.																								
REMARKS:																									
1. This chart is valid during the one, 6-hr SB 101 complete closure for the Jacoby Creek Bridge (#04-0023L) trial bridge slide and the one, 10-hr SB 101 complete closure for the Jacoby Creek Bridge (#04-0023L) final bridge slide.																									
2. A ramp detour plan must be authorized prior to the closure of any ramps.																									

Add to the end of the 1st paragraph of section 12-4.02C(7)(a):

except you may use a moving closure during traffic striping and pavement marker placement using a bituminous adhesive. Do not use a moving lane closure when grinding for recessed striping and recessed markers.

Add to the end of section 12-4.02C(7)(a):

Except where prohibited, use an impact attenuator vehicle:

- To follow behind equipment and workers who are placing and removing components of a closure. Operate the flashing arrow sign in the arrow or caution mode during this activity, whichever applies. Follow at a distance that prevents intrusion into the work space from passing traffic.
- As a shadow vehicle in a moving lane closure.

After placing components of a stationary traffic control system, you may place the impact attenuator vehicle in advance of the work area or at another authorized location to protect traffic and workers.

Add to the end of section 12-4.02C(7)(b):

Not more than 1 stationary closure is allowed in each direction of travel at one time.

Replace the 1st paragraph of section 12-6.03A with

If work activities obliterate pavement delineation, place temporary or permanent pavement delineation before opening the traveled way to traffic. The temporary pavement delineation must consist of a lane line, edge lines, and centerline pavement delineation for traveled ways open to traffic.

Add to the 1st paragraph of section 14-6.03A:

This project is within or near habitat for the regulated species shown in the following table:

Regulated Species
Tidewater goby (<i>Eucyclogobius newberryi</i>)
Southern Oregon/Northern California Coast coho salmon (<i>Oncorhynchus kisutch</i>)
California Coastal (CC) Chinook salmon ESU (<i>O. tshawytscha</i>)
Northern California steelhead (<i>O. mykiss</i>)
North American green sturgeon (<i>Acipenser medirostris</i>)
Lyngbye's sedge (<i>Carex lyngbyei</i>)

This project includes the sensitive habitats shown in the following table:

Sensitive Habitats
Jacoby Creek
Wetlands
Gannon Slough

Add to section 14-6.03A:

Species protection areas within the project limits are as specified in the following table:

Species Protection Areas	
Identification name	Location
Jacoby Creek	Jacoby Creek Bridge

Use the protocols for the corresponding regulated species shown in the following table:

Regulated species name	Protocol
Lyngbye's sedge	Notify the engineer 30 days prior to beginning construction activities

Within 50 feet of Jacoby Creek, implement the following protection measures:

1. Use biodegradable, non-toxic vegetable oil based hydraulic fluid in all equipment that requires hydraulic fluid.
2. Treated lumber is not allowed.

Replace the 2nd paragraph of section 14-6.03B with:

The Department anticipates nesting or attempted nesting by migratory and nongame birds from February 15 to September 1.

Add between the 2nd and 3rd paragraphs of section 14-6.03B:

Do not perform tree or shrub removal during nesting or attempted nesting.

Add to section 14-6.03C:

Regulated fish are anticipated adjacent to the Jacoby Creek Bridge BR. 04-0023R, Jacoby Creek Bridge BR. 04-0023L, and Gannon Slough Br. 04-0024R Implement the following protection measures:

1. Install exclusionary material, a cofferdam, or a combination of both
2. Install barrier to prevent visual disturbance to fish along both banks of Jacoby Creek through work area.
3. Provide a Contractor-supplied biologist to be present during all in-stream activities

Maintain exclusion material and cofferdams such that regulated fish are prevented from entering the work area.

Add to section 14-6.03C:

14-6.03C(1) Underwater Sound Measurement

14-6.03C(1)(a) General

14-6.03C(1)(a)(i) Summary

Section 14-6.03C(1) includes specifications to conduct, calibrate, monitor and report underwater sound measurements using a hydroacoustic system that measures and stores underwater sound levels.

14-6.03C(1)(a)(ii) Submittals

Submit a Hydroacoustic Monitoring Plan (HMP) 4 weeks prior to impact pile driving or any other activity that has the potential to produce impulsive sound waves that could be injurious to fish.

The HMP must:

1. Be prepared by a hydroacoustic monitoring specialist
2. Use the Underwater Noise Monitoring Template found at www.dot.ca.gov/env/bio/hydroacoustics.html
3. Include:
 - 3.1. Work to be performed
 - 3.2. Equipment used
 - 3.3. Duration of work
 - 3.4. Control measures
 - 3.5. Noise monitoring procedures
 - 3.6. Frequency of monitoring
 - 3.7. Positions that hydrophones would be deployed
 - 3.8. Data collection and analysis
 - 3.9. Reporting activities

Allow 14 calendar days for review.

If revisions are required, the Engineer notifies you of the date when the review stopped and provides comments. Submit a revised HMP within 10 days of receiving the comments. The Department's review resumes when a complete HMP has been resubmitted.

14-6.03C(1)(b) Quality Assurance

Do not start impact pile driving or any other activity that has the potential to produce impulsive sound waves that could be injurious to fish until the HMP is authorized.

14-6.03C(1)(c) Hydroacoustic Monitor

The hydroacoustic monitoring specialist is responsible for the implementation of the HMP.

The hydroacoustic monitoring specialist must:

1. Have at least 5 years professional experience in the field of hydroacoustic monitoring
2. Be approved by the Engineer

14-6.03C(1)(c) Equipment

Take measurements using hydrophones that have a flat frequency response and are omni-directional over a minimum frequency range of 20 Hz to 20,000 Hz.

The sound level monitoring equipment must:

1. Withstand construction environment
2. Collect signals into a data-logging device
3. Have the capability to make quality recordings using a digital audio recorder with a minimum sampling rate of 48 kHz (either solid state or tape)
4. Have an accuracy of 1 dB from 20 Hz to 20,000 Hz
5. Have the capability to measure peak sound pressures from 170 dB to 220 dB referenced to 1 micro Pascal (μPa)
6. Measure the unweighted or Z-weighted sound exposure level (SEL) in dB referenced to 1 μPa^2 -second
7. Have the capability to provide a real time readout display of underwater sound levels. The real-time display must provide the unweighted peak sound pressure and SEL
8. Log data during the required measurement event (example: one pile driving event or one day)
9. Capture the maximum peak sound pressure levels along with the RMS and SEL for each continuous 1 second period

14-6.03C(1)(d) Calibrating

Calibrate the measurement system prior to use in the field each day. An acoustical piston phone and hydrophone coupler must be used along with the manufacturer's calibration certificates.

Calibrate the measurement system in one of two ways:

1. Use an acoustically certified piston phone and hydrophone coupler that fits the hydrophone to directly calibrate the measurement system. The volume correction of the hydrophone coupler using the hydrophone is known so that the piston phone produces a known signal that can be compared against the measurement system response. The response of the measurement system is noted in the field book and applied to all measurements.
2. The procedure described above is used to calibrate a "reference" hydrophone. The reference hydrophone is then replaced with the field hydrophone used to make actual measurements. Both the field and reference hydrophones are required to have manufacturer calibration certifications that include the hydrophone sensitivities. The sensitivity of the field hydrophone will be compared with the sensitivity of the "Reference" hydrophone. The difference between the two hydrophones is the offset that will be applied to the measurements made using the "field" hydrophone. This procedure is used for different model hydrophones that do not fit the piston phone coupler. With this method, the response of the reference system to the calibration tone is noted in the field book along with the calculated "offset." The calibration is applied to all measurements made using the "field" hydrophone.

Calibrate the sound level meters (SLM) to the calibration tone prior to use in the field. The tone is then measured by the SLM and is recorded on to the beginning of the digital audio recordings that will be used. Check the system calibration status by measuring the calibration tone and recording the tones.

You are responsible for ensuring that the equipment is calibrated and set to measure sounds in the proper range. Ensure that the underwater sounds do not overload the instrumentation and the noise floor of the instrumentation is not set too high for peak levels to be measured accurately.

14-6.03C(1)(e) Monitoring

Place the hydrophones in positions identified in the HMP to determine if impulsive sound levels during construction activity are kept below the peak sound pressure level and cumulative SEL thresholds listed in the PLACs. Refer to the PLACs for additional information regarding underwater sound measurements.

Stop pile driving activity and notify the Engineer immediately if the underwater sound exceeds the peak or the cumulative SEL threshold.

Additional monitoring may be required.

14-6.03C(1)(f) Data Reporting

Record field notes in water resistant field notebooks. Notebook entries must include:

1. Operator's name
2. Date
3. Time
4. Calibration notes
5. Measurement positions
6. Noise source information
7. System gain setting
8. The name of the equipment used to make each measurement

Technicians with at least 2 years of experience in the field of hydroacoustic monitoring may perform monitoring duties under the direction of the hydroacoustic monitoring specialist. Submit the qualifications of each technician performing field monitoring. Technicians must be approved by the Engineer.

Submit hydroacoustic data daily maximum levels, mean levels and daily cumulative SEL measured.

Provide annual reports of the results from hydroacoustic monitoring no later than December 31 of each season during which monitoring occurs.

Submit a final hydroacoustic report 30 days after completion of all hydroacoustic monitoring activities. The final report must include:

1. A brief project description, methodology and presentation of results
2. Acoustical information
 - 2.1. Peak
 - 2.2. RMS
 - 2.3. SEL
 - 2.3.1. Percentage single strike or one-second SELs above 150 dB
 - 2.3.2. Maximum single strike or one-second SEL
 - 2.3.3. Mean single strike or one-second SEL above 150 dB
 - 2.3.4. Mean single strike or one-second SEL
 - 2.3.5. Daily cumulative SEL
3. Location of noise source
4. Distance from the noise source
5. Water depth/measurement depth
6. Hammer size
7. Number of strikes

14-6.03C(1)(g) Payment

Not used.

Replace the list in the 2nd paragraph of section 14-6.03D(1) with:

1. Prepare Natural Resource Protection and Education Plan.
2. Conduct environmental worker education training for all construction personal prior to the beginning of construction activities and within 5 days of any new employee beginning work after construction has begun.
3. Inspect and document ESA's and ESA fence daily to ensure no work activities are occurring within ESA.

Replace the list in the 3rd paragraph of section 14-6.03D(1) with:

Day, Time, location with stations, what was inspected for ESA, vegetation removal etc.

Replace the 2nd and 3rd paragraphs of section 14-6.03D(1) with:

The Contractor-supplied biologist must implement monitoring and reporting requirements described in the natural resources protection plan.

Add to section 14-6.03D(1):

A Contractor-supplied biologist who performs specialized activities must have demonstrated field experience working with the regulated species or performing the specialized task. The biologist must have experience that complies with the requirements shown in the following table:

Specialized activity/species	Requirements
Pacific Northwest Avian identification, biology and surveys	B.S. Degree in wildlife and fisheries biology, 3 years field experience
Experience in Wetland Delineation per USACE South Pacific Standards	Verified training and field experience (minimum 3 years)

The preconstruction survey report must include one of the following:

1. Detailed observations and locations where regulated species were observed
2. Statement that no regulated species were observed

Submit an initial monitoring report as an informational submittal within 24 hours after starting ground-disturbing activities.

Submit a biological resource incident report within 12 hours of the incident.

The incident report must include:

1. Description of any take of regulated species or any violation of a biological resource PLAC
2. Species name and number taken
3. Details of required notifications with contact information
4. Corrective actions proposed or taken
5. Disposition of taken species

Submit an annual monitoring report no later than January 15 during each year of construction.

The annual monitoring report must include:

1. Start and end dates of construction
2. Project impacts on the regulated species
3. Species protection measures and implementation details
4. Incidental take details, including species name, number taken, people contacted, contact information, and disposition of taken species
5. Assessment of the effectiveness of the species protection measures in mitigating project impacts
6. Recommendations for improving species protection measures

Submit a final monitoring report no later than 30 days after completion of the project. If the report requires revisions, the Department provides comments. Submit a revised report within 7 days of receiving comments. The final monitoring report must be a cumulative report including:

1. Start and end dates of construction
2. Project impacts on the regulated species
3. Species protection measures and implementation details
4. Incidental take details, including species name, number taken, people contacted, contact information, and disposition of taken species
5. Assessment of the effectiveness of the species protection measures in mitigating project impacts
6. Recommendations for improving species protection measures

Replace *Reserved* in section 14-6.05 with:

14-6.05 INVASIVE SPECIES CONTROL

14-6.05A General

14-6.05A(1) Summary

Section 14-6.05 includes specifications for preventing the introduction and spread of invasive species to and from the job site per Federal Executive Order 13112.

Comply with section 13-4.03E(3).

This project includes the sensitive areas shown in the following table:

Sensitive Area	
Location	Description of Area
Entire project limits	Median areas

The following invasive species are present at this job site:

Invasive Species
cotoneaster (<i>Cotoneaster pannosus</i>)
bull thistle (<i>Cirsium vulgare</i>)
poison hemlock (<i>Conium maculatum</i>)
pampas grass (<i>Cortaderia jubata</i>)
foxglove (<i>Digitalis purpurea</i>)
wild teasel (<i>Dipsacus folionum</i>)
Spanish heath (<i>Erica lusitanica</i>)
blue gum eucalyptus (<i>Eucalyptus globulus</i>)
fennel (<i>Foeniculum vulgare</i>)
French broom (<i>Genista monspessulana</i>)
English ivy (<i>Hedera helix</i>)
Klamath weed (<i>Hypericum perforatum</i>)
bird's-foot trefoil (<i>Lotus corniculatus</i>)
common reed (<i>Phragmites australis</i>)
pennyroyal (<i>Mentha pulegium</i>)
pittosporum (<i>Pittosporum sp.</i>)
Himalayan blackberry (<i>Rubus armeniacus</i>)
dense-flowered cordgrass <i>Spartina densiflora</i>)
periwinkle <i>Vinca major</i>)

14-6.05A(2) Definitions

vehicles: Construction vehicles and equipment that travel off the paved surface.

14-6.05A(2) Submittals

Prepare and submit an invasive species control plan. The plan must describe measures for preventing the introduction and spread of invasive species.

Include the following protection measures in the plan:

1. For work at the job site:
 - 1.1. At least 2 business days before using vehicles on the job site, submit a signed statement that the vehicles have been cleaned of soil, seeds, vegetative matter, and other such debris that may introduce or spread invasive species. The statement must include:
 - 1.1.1. List of the vehicles with identifying numbers
 - 1.1.2. Date of cleaning for each vehicle
 - 1.1.3. Description of the cleaning process
 - 1.1.4. Measures to be taken to ensure the vehicles remain clean until operation at the job site

- 1.1.5. Measures to be taken to ensure runoff resulting from the cleaning of vehicles will not flow into waterways, drainage inlets, or sensitive habitat.
- 1.1.6. Update the list of vehicles as needed.
- 1.1.7. Verification that the vehicles have not been operated in waters known to be infested by aquatic invasive species.
- 1.2. Designated parking areas for personal vehicles of employees.
- 2. For work in sensitive areas:
 - 2.1. Pressure wash vehicles before entering:
 - 2.1.1. At a temperature of 140 degrees F
 - 2.1.2. With a minimum nozzle pressure of 800 psi
 - 2.1.3. With a minimum fan tip angle of 45 degrees
 - 2.2. For personal equipment used in water at the job site, such as boots, waders, or hand tools, Decontaminate the equipment in accordance with: CDFW Northern Region Invasive Species Decontamination Protocols.
 - 2.3. Before exiting, ensure vehicles are free of soil and plant material.
 - 2.4. Re-clean any vehicle that is removed from the sensitive area before operating it again in the sensitive area.
- 3. Requirements of section 13-4.03E(3)

14-6.05B Materials

Not Used

14-6.05C Construction

Manage work activities to comply with the approved plan.

Personal vehicles that are parked in designated parking areas are exempt from protection measures.

14-6.05D Payment

Not used

Replace Reserved in section 14-6.06 with:

14-6.06 BAT AND BIRD EXCLUSION DEVICES

14-6.06A General

14-6.06A(1) Summary

Section 14-6.06 includes specifications for exclusion devices to prevent:

- 1. Roosting of bats.
- 2. Nesting of migratory birds and nongame birds.

Use exclusion devices at the following locations:

- 1. Temporary structures required to construct the project
- 2. Areas listed in the PLAC.
- 3. Jacoby Creek Bridge BR. 04-0023R.
- 4. Jacoby Creek Bridge BR. 04-0023L
- 5. Gannon Slough Br. 04-0024R

14-6.06A(2) Definitions

temporary structure: Protective covers, falsework, scaffolding, or similar components required to construct the project

nesting season: The dates the Department anticipates nesting or attempted nesting. Comply with Section 14-6.03B.

day roost: A roost site that bats utilize during daylight hours for resting and pup rearing including abutment joints, span hinge joints, bent joints, bridge cavities, deck drains and any access or openings to cells of box girders. Day roosting occurs April 1 through September 15.

night roost: A roost site bats use during hours of darkness for resting including any portion of a structure or components of that structure.

continuous construction presence: Actual construction activity or personnel presence, or equivalent construction noise of at least 85 dBA hourly average measured from the source to the receptor no fewer than 5 days each week for at least 8 daylight hours per day.

14-6.06A(3) Submittals

Submit an exclusion plan prepared by a qualified biologist to the Engineer. Allow 10 days for review.

Do not start jobsite activities until the plan is authorized.

The exclusion plan must include:

1. Title sheet
2. Table of contents
3. Exclusion devices to be used to exclude bats and nesting birds
4. Location and schedule of exclusion devices
5. Disposal method for partially constructed and unoccupied nests
6. Daily inspection and maintenance schedule
7. Methods of maintenance, including types of adhesive tape and/or sealants for repair, bioacoustic deterrent, and visual deterrent devices
8. PLAC requirements

14-6.06A(4) Quality Assurance

Monitor the effectiveness and maintenance of the exclusion devices as described in the PLAC.

If a nest becomes established during the nesting season:

1. Do not remove the nest.
2. Immediately contact the Engineer for evaluation and discussions of possible actions to avoid disrupting the nesting activity.

14-6.06B Materials

Materials for bird exclusion must be one or a combination of the following:

1. Polytetrafluorethylene (PTFE) sheeting.
2. Acoustical deterrent
3. Visual deterrent
4. Other materials authorized by the engineer.

You may not use devices that include netting.

Material for bat exclusion must be one or a combination of the following:

1. Backer rod
2. Expansion foam
3. Non-toxic foamed concrete (similar to Aircrete, Foamcrete or Cellular Lightweight Concrete)
4. Steel wool
5. Other materials authorized by the engineer

You may not use devices that include netting.

Exclusion devices must be installed to withstand the elements including wind and rain.

14-6.06C Construction

Install exclusion devices:

1. For temporary structures, at the time of erection.
2. For existing structures:
 - 2.1 Prior to the start of construction.
 - 2.2 During the non-nesting season and non-day roosting time period.

3. To completely block bat and bird access to the bridge or temporary structure, including its exterior girders and overhang.
4. For areas identified during the pre-construction survey for nesting of migratory and nongame birds

A qualified biologist must oversee installation, maintenance and removal of the exclusion device.

During the nesting season, nest removal is not allowed. If attempted nesting occurs during the nesting season, you may remove the nest material prior to the nests becoming one-third complete.

Clean bat and bird waste or other debris from the contact surfaces of the bridge girders before installing the exclusion devices.

Install bat exclusion devices 2 hours after sunset and when the ambient air temperature is at least 45 degrees Fahrenheit.

Monitor daily to maintain and repair devices.

Upon completion of the work, remove exclusion devices.

14-6.06D Payment

Not Used

Add to the end of section 14-9.02:

The US EPA has established the National Emission Standards for Hazardous Air Pollutants (NESHAP). Under the Health & Safety Code § 39658(b)(1), your demolition and rehabilitation activities must comply with 40 CFR 61, Subpart M (National Emission Standard for Asbestos).

The asbestos survey and sampling report for this project is included in the *Information Handout*.

Notify the US EPA and the California Air Resources Board of your demolition activities even if the activities will not disturb asbestos-containing material.

You may obtain an Asbestos NESHAP Notification of Demolition and Renovation Form at the California Air Resources Board's website:

<http://www.arb.ca.gov/enf/asbestos/asbestos.htm>

Instead of the 10 working days specified at the website, mail or deliver the form with the necessary attachments at least 15 days before starting demolition or rehabilitation activities to:

US EPA - REGION IX
ASBESTOS NESHAP NOTIFICATION (AIR-5)
75 HAWTHORNE ST
SAN FRANCISCO, CA 94105

Mail or fax a copy of the notification form to:

CALIFORNIA AIR RESOURCES BOARD
ENFORCEMENT DIVISION
ASBESTOS NESHAP NOTIFICATION
P.O. BOX 2815
SACRAMENTO, CA 95812
FAX: (916) 229-0645

Submit a copy of the notification form and attachments as informational submittals before starting demolition or rehabilitation activities.

You must notify the North Coast Unified AQMD of your demolition activities even if the activities will not disturb asbestos-containing material.

You may obtain the notification form, submittal instructions, and other information from:

North Coast Unified AQMD
707 L Street
Eureka, CA 95501
<http://www.ncuaqmd.org>

Instead of the 10 working days specified at the website, submit a notification form to the North Coast Unified AQMD at least 15 days before starting demolition or rehabilitation activities.

Submit a copy of the notification form and the necessary attachments as informational submittals before starting demolition or rehabilitation activities.

If you discover unanticipated asbestos-containing material during the demolition or rehabilitation activities, immediately stop work in that area and notify the Engineer. The Department will use other forces to remove and dispose of the material. Do not resume work in the area until authorized.

Notify the North Coast Unified AQMD of a change to your demolition or rehabilitation activities, including a revised work plan or the discovery of unanticipated asbestos-containing materials, within 2 days of the change or discovery.

Replace *Reserved* in Section 14-11.08 with:

14-11.08A General

Section 14-11.08 includes specifications for management of regulated material containing ADL. Management of the material includes:

1. Excavating
2. Loading and unloading containers or trucks
3. Transporting
4. Disposal

Manage regulated material containing ADL under the rules and regulations of the following agencies:

1. US Department of Transportation
2. US EPA
3. California Environmental Protection Agency
4. CDPH
5. DTSC
6. Cal/OSHA
7. California Department of Recycling and Recovery
8. California Air Resources Board
9. RWQCB, Region 1, North Coast
10. North Coast Unified AQMD
11. CA Coastal Commission

The Department entered into agreement Docket No. ESPO-SMA 15/16-001 Soil Management Agreement for Aerially Deposited Lead-Contaminated Soils with the DTSC (ADL Agreement) regarding the management of regulated material containing ADL. As the responsible entity and the generator of waste, only the Department determines material classification. The ADL Agreement is available at:

<http://www.dot.ca.gov/env/hazwaste/adl.html>

Regulated material containing ADL is present within the project limits and the ADL Agreement applies. Management of regulated material containing ADL exposes workers to health hazards that must be addressed in your lead compliance plan under 7-1.02K(6)(j)(ii).

14-11.08B Definitions

average ADL concentration: Average ADL concentration calculated using the 95 percent upper confidence limit.

regulated material: ADL-contaminated material that has average ADL concentrations over 80 mg/kg total lead or equal to or greater than 5 mg/L soluble lead tested using the California Waste Extraction Test (CA-WET) or equal to or greater than 5 mg/L soluble lead tested using the Toxicity Characteristic Leaching Procedure (TCLP).

Type Z-2: Regulated material that is a Department-generated California hazardous waste that must be disposed of at an appropriately permitted California Class I disposal facility. Type Z-2 material has average ADL concentrations greater than or equal to 1,000 mg/kg total lead or 5.0 mg/l soluble lead as tested using the CA-WET.

14-11.08C Site Conditions

Concentration data and sample location maps for regulated material are included in the *Information Handout*.

Type Z-2 material exists from the surface to below the existing grade as shown and listed in the following table:

Location	Depth
Location 1 – PM 84.4 to PM_84.6	0 to 1 foot

14-11.08D Submittals

14-11.08D(1) General

Not Used

14-11.08D(2) Perimeter Air Monitoring Requirements

Not Used

14-11.08D(3) Excavation and Transportation Plan

Within 10 days of Contract approval, submit 3 copies of an excavation and transportation plan for regulated material. Allow 10 days for review. If the plan requires revisions, the Department provides comments. Submit a revised plan within 7 days of receiving comments. The Engineer may allow construction to proceed while minor revisions or amendments are being completed.

The excavation and transportation plan must comply with:

1. DTSC regulations
2. ADL Agreement
3. Cal/OSHA regulations

The excavation and transportation plan must include:

1. Procedures for managing the material.
2. Excavation schedule by location and date.
3. Dust control measures.
4. Transportation equipment and routes.
5. Method for preventing spills and tracked material onto public roads.
6. Truck waiting and staging areas.
7. Name and address of the California Class I disposal facility where hazardous waste will be disposed of.
8. Spill contingency plan for regulated material containing ADL.
9. Copies of the contract plan sheets where the location and depth of the existing regulated material are shown, as an attachment.

14-11.08D(4) Burial Location Report

Not Used

14-11.08D(5) Bill of Lading

Not Used

14-11.08D(6) Disposal Documentation

Submit documentation from the receiving disposal facility confirming appropriate disposal within 5 business days of transporting Type Z-2 material from the job site.

14-11.08E Dust Control

Prevent visible dust migration under section 14-11.04 during management of regulated material.

14-11.08F Air Monitoring

Not Used

14-11.08G Stockpiling

Do not stockpile Type Z-2 material. Transfer Type Z-2 material directly from the excavation to containers or trucks for transportation to the disposal facility.

14-11.08H Placement

Not Used

14-11.08I Surveying Burial Site

Not Used

14-11.08J Material Transportation

Before traveling on public roads outside the controlled access construction zone, remove loose and extraneous regulated material from outside surfaces of containers and the cargo areas of trucks. Place tarpaulins or other cover over the cargo as described in the authorized excavation and transportation plan. You are responsible for costs due to spillage of regulated material during transport.

Transport excavated Type Z-2 material using:

1. Hazardous waste manifest
2. Hazardous waste transporter with a current DTSC registration certificate and CA Highway Patrol (CHP) Basic Inspection of Terminals (BIT) Program documentation with a satisfactory rating.

14-11.08J Disposal**14-11.08J(1) General**

Laws and regulations that govern disposal of regulated material include:

1. Health & Safety Code § 25100 et seq
2. 22 CA Code of Regs § 66250 et seq
3. 8 CA Code of Regs

The Department does not pay for additional sampling and analysis required by disposal facilities.

14-11.08J(2) Type Com Material

Not Used

Replace *Reserved* in section 14-11.09 with:

14-11.09A General

Section 14-11.09 includes specifications for handling and managing regulated material containing ADL when there is a minimal disturbance. Regulated material containing ADL has average ADL concentrations over 80 mg/kg total lead or equal to or greater than 5 mg/L soluble lead tested using the California Waste Extraction Test or equal to or greater than 5 mg/L soluble lead tested using the toxicity characteristic leaching procedure.

Compliance with 22 CA Code of Regs is not required where there is minimal disturbance of regulated material containing ADL.

Management of regulated material containing ADL exposes workers to health hazards that must be addressed in your lead compliance plan.

Handle regulated material containing ADL under the rules and regulations of the following agencies:

1. Cal/OSHA
2. RWQCB, Region 1 — North Coast
3. North Coast Unified Air Quality Management District
4. CA Coastal Commission

Regulated material containing ADL is typically found within the top 2 feet of material in ADL impacted areas of the job site. Concentrations of ADL found in the area of minimal disturbance range from 4 to 160 mg/kg total lead. Lead concentrations were analyzed by US EPA Method 6010 or US EPA Method 7000 series.

Minimal disturbance of regulated material containing ADL occurs where the following work activities are conducted:

1. Guardrail and post removal and installation
2. Roadside sign post removal and installation

14-11.09B Material Management

Handling of regulated material containing ADL must result in no visible dust migration. Use dust control measures. A means of controlling dust must be available at all times.

Separate material from vegetation. The resulting soil must remain on the job site.

Surplus material from the areas with regulated material containing ADL must remain in the area of disturbance. Do not dispose of surplus material outside the highway.

Add after the 2nd paragraph of section 14-11.12A:

This project includes removal of yellow painted traffic stripe that will produce hazardous waste residue.

Add after the 1st paragraph of 14-11.12E:

After the Engineer accepts the analytical test results, dispose of yellow thermoplastic and yellow paint hazardous waste residue at a Class 1 disposal facility located in California 60 days after accumulating 220 lb of residue.

If less than 220 lb of hazardous waste residue and dust is generated in total, dispose of it within 60 days after the start of accumulation of the residue.

Add to the 1st paragraph of section 14-11.14A:

Wood removed from guardrail and roadside sign posts is treated wood waste.

Replace section 14-11.16 with:

14-11.16 ASBESTOS-CONTAINING CONSTRUCTION MATERIALS IN BRIDGES

14-11.16A General

Section 14-11.16 includes specifications for removing and managing asbestos-containing construction materials (ACCM) in bridges.

This project involves the removal and management of ACCM.

The removal and management of asbestos must comply with:

1. Health and Safety Code Div 20 Ch 6.5, Hazardous Waste Control
2. 8 CA Code of Regs § 5208
3. 8 CA Code of Regs §§ 1529 and 341.6–341.17
4. 22 CA Code of Regs Div 4.5

5. 29 CFR 1926
6. 40 CFR 61 Subpart M - National Emissions Standard for Asbestos
7. Bus & Prof Code §§ 7058.5–7058.6, 7180-7189.7, and 7028.1

Friable ACM generated as part of this project is a Department-generated hazardous waste as specified in section 14-11.07.

14-11.16B Definitions

asbestos: Any of several minerals that readily separate into long flexible fibers. Includes chrysotile, amosite, crocidolite, tremolite, anthrophyllite, actinolite, and any of these minerals that has been chemically treated, altered, or both.

asbestos-containing construction materials (ACCM): Manufactured construction material which contains more than 1/10th of 1 percent asbestos by weight under 8 CA Code of Regs § 341.6.

asbestos-containing material (ACM): Building material, including asbestos cement pipe and concrete, containing more than 1 percent asbestos by weight, area, or count under 40 CFR § 61.145.

Category I nonfriable ACM: Asbestos-containing packings, gaskets, resilient floor covering, and asphalt roofing products containing more than 1 percent asbestos under 40 CFR § 61.141.

Category II nonfriable ACM: Any material, excluding Category I nonfriable ACM, containing more than 1 percent asbestos that, when dry, cannot be crumbled, pulverized, or reduced to powder by hand pressure under 40 CFR § 61.141.

certified asbestos consultant: Asbestos consultant certified by Cal/OSHA under 8 CA Code of Regs §§ 341.15 and 1529. A certified asbestos consultant must be registered or working for a company registered under Labor Code § 6501.5 and certified under Bus & Prof Code § 7058.6.

friable ACM: Material containing more than 1 percent asbestos, as determined by polarized light microscopy, that can be crumbled, pulverized, or reduced to powder by hand pressure when dry under 22 CCR § 66261.24.

nonfriable ACM: Material containing more than 1 percent asbestos by area with asbestos fibers that:

1. Are tightly bound into the matrix of the material
2. Should not become an airborne hazard as long as the material remains intact and undamaged and is not sawed, sanded, drilled, or otherwise abraded during removal

nonhazardous asbestos waste: ACCM with an asbestos concentration less than 1 percent or nonfriable ACM. These wastes are not hazardous wastes under 22 CA Code of Regs Div 4.5.

regulated asbestos-containing material (RACM): Under 40 CFR § 61.141, RACM is defined as any of the following:

1. Friable ACM
2. Category I nonfriable ACM that has become friable or will be or has been subjected to sanding, grinding, cutting, or abrading
3. Category II nonfriable ACM that may become, has become, or has a high probability of becoming friable

14-11.16C Site Conditions

An asbestos survey was performed for bridge no. 04-0023L/R and 04-0024L/R.

The relevant portions of the asbestos survey report are included in the *Information Handout*. ACCM is present and will be disturbed by work at the locations and in the types and amounts shown in the following table:

Bridge location	Asbestos location	Category of asbestos	Friable or nonfriable	Percent asbestos	Amount of asbestos
04-0023L	Bent column 2	Category I	Nonfriable	Non-Hazardous	24 SF
04-0024L	Bent column 4, 7, 10, 13	Category I	Nonfriable	Non-Hazardous	96 SF

14-11.16D Submittals

14-11.16D(1) General

Not Used

14-11.16D(2) Asbestos Compliance Plan

Submit an asbestos compliance plan for preventing or minimizing workers' exposure to asbestos during demolition or renovation activities. Submit the plan at least 15 days before starting bridge demolition or renovation activities in areas containing or suspected to contain asbestos. The plan must be prepared and signed and sealed by a CIH with experience and knowledge of asbestos removal work and by the certified asbestos consultant who will direct the removal, storage, transportation, and disposal of ACM. The plan must include:

1. Identification of key personnel for the project
2. Scope of work and equipment to be used
3. Job hazard analysis for work assignments
4. Summary of risk assessment
5. Description of personal protective equipment
6. Delineation of work zones at the job site
7. Decontamination procedures
8. General safe work practices
9. Security measures
10. Emergency response plans
11. Safety training program

14-11.16D(3) Asbestos Removal Work Plan

Submit a work plan for the removal and management of asbestos 15 days before starting bridge demolition or renovation activities in areas containing or suspected to contain asbestos. The work plan must be prepared and signed and sealed by a certified asbestos consultant and include:

1. Name of the certified asbestos consultant who will direct the removal, storage, transportation, and disposal of ACM.
2. Locations at the perimeters of abatement work areas where asbestos warning signs will be installed.
3. Summary of the methods and techniques for removal, handling, packaging, labeling, storing, transporting, and disposing of waste materials.
4. Instructions for wetting asbestos materials with sprayers.
5. Description and locations of disposal bins for temporary storage of asbestos until removal from the job site.
6. Name and address of the hazardous waste transporter that will transport friable ACM. The transporter must be registered with the DTSC to transport hazardous waste under the Health and Safety Code Div 20 Ch 6.5 and 22 CA Code of Regs Div 4.5.
7. Name and address of the California disposal facility permitted for the disposal of ACM.
8. Documentation of compliance with federal, State, and local requirements for asbestos work, transport, and disposal.

14-11.16D(4) Certification of Completed Safety Training

Submit certification of completed safety training for all personnel before starting work in areas containing or suspected to contain asbestos.

14-11.16D(5) Asbestos Removal Report

Submit an asbestos removal report documenting your compliance with the asbestos removal work plan. Submit the report to the Engineer and the APCD or AQMD within 30 days after removing ACM from the job site.

14-11.16D(6) Disposal Documentation

Submit a copy of the hazardous waste manifest for each shipment of friable ACM. Submit a copy of the waste shipment record for each shipment of nonhazardous asbestos waste.

Within 5 business days of transporting hazardous and nonhazardous asbestos waste, submit documentation of proper disposal from the receiving disposal facility.

14-11.16E Health and Safety

Before starting work in areas containing or suspected to contain asbestos, provide safety training complying with 8 CA Code of Regs § 1529 to State employees who may enter the work area.

Provide training, personal protective equipment, and medical surveillance as required by the asbestos compliance plan for 3 State employees.

14-11.16F Removal and Disposal of Unanticipated Asbestos

If you discover unanticipated asbestos during demolition or rehabilitation activities, immediately stop work in that area and notify the Engineer.

The removal and disposal of ACM not identified in the asbestos survey report is change order work.

14-11.16G Removal of Asbestos

Remove asbestos under 8 CA Code of Regs § 1529 and 341 et seq. Remove friable asbestos using the wetting method. Remove and handle nonfriable asbestos such that you prevent breakage.

You are not required to remove asbestos encased in concrete or similar structural material before starting demolition. Keep the asbestos wet whenever it is exposed during demolition activities. Prevent airborne emissions from asbestos removal activities.

Mark the regulated work areas with warning signs that read, *Danger, Asbestos, Cancer and Lung Disease Hazard, Authorized Personnel Only*. The message must be legible from a distance of 20 feet by persons with 20/20 vision or vision corrected to 20/20.

14-11.16H Packaging and Temporary Storage of Asbestos-Containing Material

Package and label removed ACM under 22 CA Code of Regs § 66262.30 et seq. Place the removed ACM in minimum, 0.006-inch-thick, double-ply, plastic bags with clearly visible and legible labels affixed to the bags. The labels must read, *Danger/ Contains Asbestos Fibers/ Avoid Creating Dust/ Cancer and Lung Disease Hazard*. Wet the waste before placing it in the plastic bag to prevent asbestos fibers from becoming airborne if the bag is broken.

Do not break apart bulk waste that will not fit inside a plastic bag. Instead, wet the waste, wrap it in plastic, and seal it with packaging or duct tape until it is leak-proof. Place the wrapped and sealed ACM directly into a covered, lockable, roll-off or drop box lined with plastic sheeting and labeled on all sides. The labels must be legible and read, *Danger/ Contains Asbestos Fibers/ Avoid Creating Dust/ Cancer and Lung Disease Hazard*.

14-11.16I Transport and Disposal of Asbestos-Containing Construction Materials

14-11.16I(1) General

Dispose of ACCM at a California disposal facility operating under a RWQCB permit to accept asbestos waste. Notify the facility at least 5 business days before the delivery of ACCM.

14-11.16I(2) Friable Asbestos-Containing Material

Transport and dispose of friable ACM as a hazardous waste. The Engineer provides the Department's EPA Identification Number for hazardous waste disposal. The Engineer signs the hazardous waste manifests. Notify the Engineer 5 business days before the manifests are to be signed.

Use a transporter for friable ACM with:

1. Current DTSC registration for transporting hazardous waste
2. US EPA Identification number
3. Proof of completion of the California Highway Patrol's Basic Inspection of Terminals Program with a satisfactory rating

The transporter's vehicles must carry a valid DTSC registration when transporting friable ACM.

14-11.16I(3) Nonhazardous Asbestos Waste

Transport nonhazardous asbestos waste to the disposal facility with a waste shipment record.

Add to the end of section 14-12.01:

Submit to the Engineer PLAC required:

1. Communications. The Engineer will make all contact with the agencies.
2. Submittals. The contractor will allow for a 60 day review period for all PLAC submittals unless a shorter review period is allowed by the engineer.
3. Records to be maintained, within 5 days after the triggering activity.

Replace *Reserved* in section 14-12.04 with:

14-12.04 PLAC RESPONSIBILITY AND CLARIFICATION

14-12.04A General

Section 14-12.04 contains specifications relating to PLAC commitments.

14-12.04B Materials

Not Used.

14-12.04C Construction

Perform all work described in the PLACs on behalf of the Department unless listed in the following table:

Work to be performed by the Department	
PLAC	Description
Coastal Development Permit # 1-18-1078	Condition 11 : The Department will develop and submit an on-site mitigation and monitoring plan
Coastal Development Permit # 1-18-1078	Condition 15: The Department will develop and submit a final lighting plan.
Coastal Development Permit # 1-18-1078	Condition 20: The Department will develop and submit a corridor tree restoration planting plan.
Coastal Development Permit # 1-18-1078	Condition 24: The Department will provide the Coastal Commission the Army Corps of Engineers permit.
Coastal Development Permit # 1-18-1078	The Department will develop and submit a corridor tree restoration planting plan.
California Fish and Wildlife Service Permit #1600-2018-0503-R1	Condition 2.9: The Department will perform Pre-Project Bat Surveys.

The following clarifications are provided for the PLAC's:

Clarification of PLAC requirements	
PLAC	Description
Coastal Development Permit # 1-18-1078	Condition 18: The Department is responsible for parts A, C, D, and F. The Contractor is responsible for part B.

14-12.04D Payment

Not Used.

Fiber Reinforced Matrix

Quality Characteristic	Test method	Requirement
Color	Observed	Colored to contrast application area, must not stain concrete painted surfaces or HMA
Organic Matter Content (min. %)	ASTM D2974	90
Minimum Water Holding Capacity (%)	ASTM D7367	700
Acute Toxicity	ASTM 7101 EPA 2021.0-1	Non-Toxic
Functional Longevity (Days)	Department Approved Testing Method	365
Maximum Slope Application (H:V)	Observed	1.0:1.0
Rainfall Event (R-factor)	ASTM D6459	160<R
Cover Factor	ASTM D6459	C ≤ 0.01
Functional Longevity (min Months)	ASTM D5338	12
Minimum Vegetation Establishment (%)	ASTM D7322	500

FRM must be one of the products below or an approved equal.

Product	Manufacturer	Contact
Flexterra™ HP-FGM CocoFlex™ ET-FGM	Profile Products	750 W. Lake Cook Rd, Suite 440 Buffalo Grove, IL 60089 800-508-8681 http://www.profileevs.com
FlexGuard®	Mat, Inc	12402 Hwy 2 Floodwood, Min 55736 888-477-3028 http://www.matinc.biz
Hydra CX2 Extreme Slope Matrix	North American Green	5401 St. Wendel-Cynthiana Road Poseyville, IN 47633 (800) 772-2040 https://nagreen.com
HY-C4	East Coast Erosion Blankets	443 Bricker Rd. Bernville, PA 19506 800-582-4005 http://www.eastcoasterosion.com

Replace *Reserved* in section 21-2.03K with:

21-2.03K Fiber Reinforced Matrix

Apply FRM with hydraulic spray equipment.

Add water to FRM as recommended by the manufacturer and mix sufficiently to ensure an even application. A dispersing agent may be added to the mixture if authorized.

Equipment must have a built-in continuous agitation and discharge system capable of producing a homogeneous mixture and uniform application rate. The tank must have a minimum capacity of 1,000 gallons. You may use a smaller tank if authorized.

Apply FRM in the locations and at the rates shown and as follows:

Delete section 39-2.01C(4)(b).

Add to section 39-2.02A(1):

Do not place Type A HMA on the traveled way from November 1 to May 1.

Add to the table in the 1st paragraph of section 39-2.02A(4)(b)(ii):

Coarse durability index ^e , D _c	AASHTO T 210	1 per 3,000 tons or 1 per paving day, whichever is greater
Fine durability index, D _f	AASHTO T 210	1 per 3,000 tons or 1 per paving day, whichever is greater
Sodium sulfate soundness (max loss @ 5 cycles, %) ^f	AASHTO T 104	1 per project

^eThe test is required only if the aggregate source is in Lassen, Modoc, Siskiyou, or Shasta County.

^fThe test is required only if the aggregate source is in Modoc, Siskiyou, or Shasta County.

Replace *Reserved* in section 39-2.02B(3) with:

The grade of asphalt binder for Type A HMA must be PG 64-28M.

For Type A HMA using RAP substitution of greater than 15 percent of the aggregate blend, the virgin binder grade must comply with the PG binder grade specified above with 6 degrees C reduction in the upper and lower temperature classification.

For Type A HMA using RAP substitution of 15 percent or less of the aggregate blend, the grade of the virgin binder must comply with the PG binder grade specified above.

Add to the beginning of section 39-2.02C:

Use a material transfer vehicle when placing Type A HMA if:

1. Quantity of HMA to be paved is greater than 1,000 tons.
2. Any of the following exists:
 - 2.1. Paving is allowed and the ambient air temperature is below 70 degrees F.
 - 2.2. Time from discharge to truck at the HMA plant until transfer to the paver's hopper is 90 minutes or greater.

Add to the end of the 1st paragraph in section 39-2.04A(1):

HMA-O includes hot mix asphalt - open graded colorized (open graded friction course) (HMA-OC).

Replace the 2nd paragraph in section 39-2.04A(1) with:

Produce OGFC using a WMA additive technology.

Add to the end of section 39-2.04A(3):

For HMA-OC, in the QC plan, submit the following for the integral color:

1. Technical data sheet
2. Manufacturer's specifications
3. Manufacturer's recommendation for producing the required integral color in HMA

7. Remove the cables and instruments from the monitored pile and deliver them to the Engineer.
8. After 4 days of driving the pile to within 1 foot of specified tip elevation, install the instrument package on the pile again and attach the cables and resume driving the pile to the specified tip elevation.
9. Remove the cables and instruments from the monitored pile and deliver them to the Engineer.
Replace in kind any cables or instruments that are damaged by your activities.

Add to the end of section 49-1.01D(4):

The Department performs dynamic monitoring of the first production pile driven for each control zone at the support location shown in the following table:

Bridge no.	Control zone	Dynamic monitoring support location
04-0313L	Abutment 1	Abutment 1
04-0313L	Abutment 2	Abutment 2

Add to section 49-1.03:

Expect difficult pile installation due to the conditions shown in the following table:

Pile location		Conditions
Bridge no.	Support location	
04-0313L	Abutments 1 & 2	Presence of ground water

Add to section 49-2.01A(3)(b):

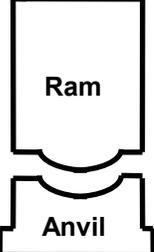
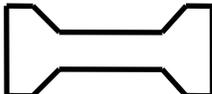
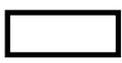
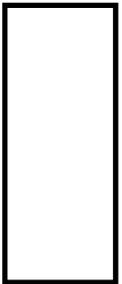
Before installing driven piles, submit a driving system submittal for each pile type for each of the support locations or control zones shown in the following table:

Bridge no.	Pile type	Support location or control zone
04-0313L	36" Cast-In-Steel Shell	Abutments 1 & 2

CALIFORNIA DEPARTMENT OF TRANSPORTATION
 TRANSPORTATION LABORATORY

PILE AND DRIVING DATA FORM

Structure Name : _____ Contract No.: _____
 _____ Project: _____
 Structure No.: _____ Pile Driving Contractor or
 Dist./Co./Rte./Post Mi: _____ Subcontractor _____ (Pile Driven By)

 Ram Anvil	Hammer	Manufacturer: _____ Model: _____ Type: _____ Serial No.: _____ Min Rated Energy: _____ at _____ Length of Stroke _____ Fuel Setting _____ Max Rated Energy: _____ at _____ Length of Stroke _____ Fuel Setting _____ Ram Weight: _____ kips Modifications: _____ _____ _____				
	Capblock (Hammer Cushion)	Material: _____ Thickness: _____ in Area: _____ in ² Modulus of Elasticity - E: _____ ksi Coefficient of Restitution - e: _____				
	Pile Cap	<table border="1" style="display: inline-table; vertical-align: middle;"> <tr><td>Helmet</td></tr> <tr><td>Bonnet</td></tr> <tr><td>Anvil Block</td></tr> <tr><td>Drivehead</td></tr> </table> Weight: _____ kips	Helmet	Bonnet	Anvil Block	Drivehead
Helmet						
Bonnet						
Anvil Block						
Drivehead						
	Pile Cushion	Material: _____ Thickness: _____ in Area: _____ in ² Modulus of Elasticity - E: _____ ksi Coefficient of Restitution - e: _____				
	Pile	Pile Type: _____ Length (In Leads): _____ ft Lb/ft.: _____ Taper: _____ Wall Thickness: _____ in Cross Sectional Area: _____ in ² Design Pile Capacity: _____ kips Description of Splice: _____ _____ Tip Treatment Description: _____ _____ _____				

DISTRIBUTE:

Translab,
Foundation Testing

Translab,
Geotechnical Design

Resident Engineer

Note: If mandrel or follower is used to drive the pile, attach separate manufacturer's detail sheet(s) including weight and dimensions.

Submitted By: _____
 Date: _____ Phone No.: _____

Replace item 4 in the list in the 3rd paragraph of section 49-3.03C(2) with:

- 4. Bottom of the pile must not be cleaned out. The top of soil plug must be at an elevation of -40 feet.

Replace the 6th paragraph of section 49-3.03C(2) with:

Dewater the steel shells before placing reinforcement and concrete. Seal the bottom of the steel shell under section 51-1.03D(3), except a minimum seal course thickness of 5 feet must be placed in the steel shells. After sealing, dewater and clean out the steel shell. During the removal of the soil plug and during placement of the seal course, maintain a positive piezometric fluid head inside the steel shell.

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51 CONCRETE STRUCTURES

Add to section 51-1.01A:

After the bridge superstructure for Bridge No. 04-0313L has been moved into its final location, grind bridge deck as directed by the Engineer, to conform with the profile grade and superelevation of the approach slabs. Grinding must conform to section 42-3.

Concrete for bridge deck, overhang, and barrier slabs at Bridge nos. 04-0023R and 04-0024R must be constructed using RSC.

Add to the list in the 1st paragraph of section 51-1.01C(4):

- 8. Penetration or slump test results
- 9. Concrete density test results
- 10. Shrinkage test results for bridge decks

Add to the end of section 51-1.01C(4):

Submit an RSC placement work plan which includes the methods and procedures for:

- 1. Storing, handling, and transporting
- 2. Staging the materials at the job site
- 3. Mixing
- 4. QC sampling and testing
- 5. Placing, finishing, and curing

Submit a contingency plan for correcting problems during transportation, production, placement, or finishing of RSC.

Submit an RSC placement work plan for mock-up construction for RSC bridge deck overhang and RSC bridge deck.

RSC placement work plans must be authorized before starting related work.

Submit compressive strength test results from the mock-up.

Replace *Reserved* in section 51-1.01D(2)(b)(i) with:

Provide a QC manager to administer the QC plan. The QC manager must hold current ACI certification as a Concrete Field Testing Technician-Grade I and a Concrete Laboratory Testing Technician-Grade II.

The QC manager must review and sign the sampling, inspection, and test reports before submitting them. The QC manager must be present for:

1. Establishing volumetric mixer control settings
2. Mock-up construction
3. Meetings with the Engineer relating to production, placement, or testing

The QC manager must not be a member of this project's production or paving crews, an inspector, or a tester. The QC manager must have no duties during the production and placement of RSC except those specified.

Schedule and hold a preconstruction meeting at least 5 business days before RSC work. You must provide the meeting facility.

The meeting must include the Engineer, your representatives, and any subcontractors involved in RSC work.

The purpose of this meeting is to establish contacts and communication protocol between you and your representatives, any subcontractors, and the Engineer.

The Engineer conducts the meeting. Be prepared to discuss:

1. Storing and staging materials at the job site
2. QC sampling and testing
3. Mixing, placing, consolidating, finishing, and curing
4. Contingency plan

Add between the 1st and 2nd paragraphs of section 51-1.01D(2)(b)(ii):

The RSC for constructing the mock-up may be used to concurrently prequalify the RSC mix design.

Replace *Reserved* in section 51-1.01D(2)(b)(iii) with:

Construct a mock-up for bridge deck overhangs and bridge decks to be constructed with RSC.

The mock-up must be 10 feet long by 10 feet wide with shown thickness and similar to the "Deck and Overhang RSC Mock-up" shown..

Construct the mock-up using the personnel, materials, tools, equipment, and methods to be used in the RSC work.

The mock-up forms must be similar to the forms used for the production element. Include in the mock-up the RSC, reinforcement (existing and new), anchor bolts, nuts, drill and bond dowels, and concrete embedments shown.

Formed surfaces for the mock-up must conform to section 51-1.03C(2).

The mock-up must demonstrate:

1. RSC encapsulates the reinforcement and embedments and complies with section 51-1.03D.
2. You are capable of constructing a bridge element within the anticipated time periods, including delivery, placement, finishing, curing times, and in similar atmospheric conditions expected during construction.

Place RSC in the mock-up in the Engineer's presence.

Test the RSC for compressive strength under section 90-1.01D(5). Prepare the cylinders under California Test 540, Method 2. RSC for mock-up construction must develop the following minimum compressive strengths:

1. 3,250 psi at age of break
2. 5,000 psi at 28 days

Saw cut the mock-up full-depth as directed.

If the Engineer rejects the mock-up, construct additional mock-ups until the mock-up is authorized. The mock-up must be authorized before starting RSC work.

Dispose of the mock-up.

Add to section 51-1.02B:

Concrete for concrete bridge decks must contain polymer fibers. Each cubic yard of concrete must contain at least 1 pound of microfibers and at least 3 pounds of macrofibers.

Concrete for concrete bridge decks must contain a shrinkage reducing chemical admixture. Each cubic yard of concrete must contain at least 3/4 gallon of a shrinkage reducing admixture. If you use the maximum dosage rate shown on the Authorized Material List for the shrinkage reducing admixture, your submitted shrinkage test data does not need to meet the shrinkage limitation specified.

Replace the paragraphs of 51-1.02D with:

RSC used to construct bridge decks and overhangs must have a minimum specified compressive strength of 3,250 psi at the age of break.

If you use chemical admixtures or SCMs in RSC, the same proportions must be used when testing.

If your RSC shrinkage test results are 0.024 percent or less without the use of a shrinkage reducing chemical admixture:

1. Use of shrinkage reducing chemical admixture is not required
2. Fibers are not required

For bridge decks, if you use the maximum dosage rate shown on the Authorized Material List for shrinkage reducing admixture, your shrinkage test results must be 0.032 percent or less.

Aggregate must be on the Authorized Material List for innocuous aggregate or your proposed mix design must comply with one of the following:

1. Any hydraulic cement, with or without any proposed SCM, must have an expansion ratio of less than 0.10 percent when tested with glass aggregate under ASTM C1260. Test specimens must be prepared using proportions of ingredients under ASTM C441.
2. For portland cement, the quantity of SCM in your proposed mix design must satisfy equation 1 of section 90-1.02B(3).

The specifications for a reduction in the operating range and contract compliance for cleanness value and sand equivalent specified in section 90-1.02C(2) and section 90-1.02C(3) for aggregate do not apply to RSC used for a bridge element.

Replace the 2nd paragraph of section 51-1.03H with:

Cure the top surface of bridge decks by (1) misting and (2) the water method using a curing medium under section 90-1.03B(2). After strike off, immediately and continuously mist the deck with an atomizing nozzle that forms a mist and not a spray. Continue misting until the curing medium has been placed and the application of water for the water method has started. At the end of the curing period, remove the curing medium and apply curing compound on the top surface of the bridge deck during the same work shift under section 90-1.03B(3). The curing compound must be curing compound no. 1.

Delete the 4th paragraph of section 51-1.03H.

Add to section 51-1.04:

The payment quantity for seal course concrete does not include the volume of seal course concrete in the cast-in-steel shell piles.

Replace item 4 in the list in the 2nd paragraph of section 51-4.03B with:

4. For box girders, a minimum of 1.5 inch of deck slab concrete is maintained between the deck slab reinforcement and the top of expanded polystyrene in the area between the girder webs:

Replace the 1st paragraph in section 51-5.01A with:

Section 51-5 includes specifications for constructing approach slabs, paving notch extensions, and barrier slabs.

Replace "*approach slabs*" in the 1st and 3rd paragraphs in section 51-5.01D(2)(b) with "*approach slabs or barrier slabs*"

Add to the end of section 51-5.01A:

If RSC mix design authorized for RSC bridge decks and RSC bridge deck overhangs is used for approach slabs and barrier slabs, the requirements for trial slab in this section are waived.

If RSC mix design authorized for RSC bridge decks and RSC bridge deck overhangs is used only for either the approach slabs or barrier slabs, then requirements for trial slab in this section are waived for that element only.

Add between 1st and 2nd paragraphs in section 51-5.02C:

Concrete for barrier slabs must contain at least 675 pounds of cementitious material per cubic yard and comply with the specifications for RSC.

Replace "*approach slabs and paving notch extensions*" in the 4th paragraph in section 51-5.02C with "*approach slabs, barrier slabs, and paving notch extensions*"

Replace "*approach slabs*" in the 2nd and 3rd paragraphs in section 51-5.03 with "*approach slabs and barrier slabs*"

Add to section 51-5.03:

51-5.03F Barrier Slabs

51-5.03F(1) General

Barrier slabs work includes removing existing facilities and constructing barrier slab.

51-5.03F(2) Removal of Existing Facilities

Remove portions of existing pavement and base, asphalt concrete surfacing, subsealing material, and cement-treated base, as necessary for the construction of the new barrier slab.

Before removing asphalt concrete, cut the outlines of excavations in asphalt concrete on a neat line to a minimum depth of 0.25 foot using a power-driven concrete saw or wheel-type rock-cutting excavator. Remove the asphalt concrete without damaging the surfacing to remain in place.

51-5.03F(3) Existing Base Material

Uniformly grade and compact the existing base material remaining in place after removing the existing pavement and base materials to the required depth. The finished surface of the base material at any point must not extend above the authorized grade.

51-5.03F(4) Profile Grade

Establish a grade line for the new barrier slab that will provide a smooth profile grade. The profile grade must be authorized.

AA

52 REINFORCEMENT

Add to section 52-2.01A(3):

52-2.01A(3)(c) Certificates

Submit a certificate of compliance for each shipment of dual-coated bar reinforcing steel. Include the following with the submittal:

- 1. Certification that the reinforcement complies with ASTM A1055
- 2. All certifications specified in ASTM A1055

Add to section 52-2.01B:

You may use dual-coated bar reinforcing steel complying with ASTM A1055 as an alternative to epoxy-coated reinforcement or epoxy-coated prefabricated reinforcement. Bar reinforcing steel to be dual-coated must be deformed, Grade 60 bars complying with ASTM A706.

Dual-coated bar reinforcement must be the same bar size and must be placed at the same spacing as described for epoxy-coated reinforcement and epoxy-coated prefabricated reinforcement.

Add to section 52-2.01C:

Do not bend bar reinforcing steel complying with ASTM A1055 after coating application if used as an alternative to epoxy coated prefabricated reinforcement.

Job site and PC plant practices for substituted bar reinforcement must comply with appendix X1 of ASTM A1055, except replace "should" with "must."

Add to section 52-2.03A(1):

Epoxy coat all reinforcement, except the reinforcement inside the CISS piling and median abutment backwalls, at the following locations:

- 1. Bridge No. 04-0313L
- 2. Bridge No. 04-0024R
- 3. Bridge No. 04-0023R

AA

55 STEEL STRUCTURES

Add to section 55-1.01A:

Timber for shear keys at median bridge must comply with section 57-2.

AA

60 EXISTING STRUCTURES

Add to section 60-2.01A:

Remove the following structures or portions of structures:

Bridge no./Structure name	Description of work
04-0023R	Top of wingwall, bridge deck, and curb
04-0024R	Top of wingwall, bridge deck, and curb
04-0023L	Entire bridge and median abutment backwalls

Payment for removing median shear keys is not included in the payment for bridge removal.

During removal of the median abutment backwall, remove the backwall reinforcement to 1 inch below the top of the median abutment to be left in place. During removal of the wingwall, remove the wingwall reinforcement to 1 inch below the top of the wingwall to be left in place. Patch holes with rapid setting concrete complying with section 60-3.02.

Replace the 2nd paragraph in section 60-2.01C with

Remove piling, piers, abutments, footings, and pedestals to 1 foot below the ground line or 3 feet below finished grade, whichever is lower, except removal of piers at bridge no. 04-0313L must end at 1 foot above the ground line.

Add to section 60-2.02A(3):

For the following bridges or portions of bridges, allow the days shown in the following table for the review of the bridge removal work plan:

Bridge or portion of bridge	Review time (days)
Jacoby Creek Bridge (Left) (Replace) Bridge No. 04-0313L	60

Add to the list in the 1st paragraph of section 60-2.02C(2):

- 6. Falsework or supports for protective covers must not extend below the vertical clearance level or to the ground line at any location within the river bed.

Replace section 60-5.03 with:

60-5.03 PLUG DECK DRAIN

60-5.03A General

Section 60-5.03 includes specifications for plugging existing deck drains.

Rapid setting concrete must comply with section 60-3.02.

60-5.03B Materials

Not Used

60-5.03C Construction

Removing portion of bridge deck, galvanized pipe, and existing drain bar grate must comply with section 60-2.

Clean surface of down drains and remove all laitance, surface contaminants, and foreign material from the deck drains.

Seal bottom of the down drains to form water tight connection.

Fill the drain and voids in the drain area and patch deck with rapid setting concrete.

60-5.03D Payment

Not used

Replace section 60-6 with:

60-6 BRIDGE SUPERSTRUCTURE MOVE

60-6.01 GENERAL

60-6.01A Summary

Section 60-6 includes specifications for moving the new superstructure of the Jacoby Creek Bridge (Left), Bridge No.04-0313L, into its final position in a safe and controlled manner.

Do not damage the median abutments, permanent abutments, and the bridge superstructure before, during, and after the bridge superstructure move.

You must:

1. Design, furnish, construct, monitor, maintain, and remove bridge superstructure move system.
2. Conduct a trial test bridge superstructure move of at least 6-inches towards the permanent abutment and back before the actual move
3. Remove and reinstall temporary shear keys at the median abutments as required for the trial and actual bridge superstructure moves.
4. Perform the bridge superstructure moves.
5. Perform interim and post-move inspections and any necessary remedial actions.

Furnish and maintain stand-by critical pieces of equipment needed for bridge superstructure move.

Equipment loads for bridge superstructure move must be supported from the median and permanent abutments. Pile driving activities for bridge superstructure move operations are not allowed.

Temporary supports and jacking system must comply with the specifications for falsework in section 48-2.02B(4).

60-6.01B Definitions

temporary supports: All components of temporary structures erected to support construction activities to erect, jack, and temporarily support the new superstructure.

deflection: Difference in vertical elevation from the required elevation.

lateral alignment: Alignment of the bridge superstructure perpendicular to the length of the bridge.

longitudinal alignment: Alignment of the bridge superstructure along the length of the bridge.

twist: Measured as the upward or downward deflection of one corner relative to the plane defined concurrently by the elevations of the other corners.

bridge superstructure move system: All components including temporary supports, jacks, tracks, rollers, sliding pads, pumps, and programmable logic controller (PLC) equipment required to temporarily support, jack, and move the new superstructure from the median abutment to its final position.

geometric monitoring: Monitoring the lateral and longitudinal alignments and twist of the bridge superstructure.

displacement monitoring: Monitoring the deflection of the bridge superstructure.

geometric control system: All components related to displacement monitoring and geometric monitoring of the new superstructure during all phases of construction.

60-6.01C Submittals

60-6.01C(1) General

Submit the shop drawings and work plans specified for the bridge superstructure move concurrently.

60-6.01C(2) Contractor Qualifications

Submit the following documentation at least 15 days before the pre-bridge superstructure move meeting:

1. Summary of the bridge superstructure move Contractor's experience that demonstrates compliance with section 60-6
2. List of at least 3 projects completed in the last 5 years that demonstrate the bridge superstructure move Contractor's ability to perform bridge superstructure moves similar to Jacoby Creek Bridge (Left). For each project, include:
 - 2.1. Project description
 - 2.2. Name and phone number of the owner
 - 2.3. Bridge superstructure move start and completion dates
 - 2.4. Shop drawings including constructions details, structural details, geometric control monitoring system, and assembly details

60-6.01C(3) Shop Drawings

60-6.01C(3)(a) General

Submit the shop drawings to OSD, Documents Unit. Notify the Engineer of the submittal. Include the date and list of contents in the submittal.

Allow 45 days for review.

60-6.01C(3)(b) Bridge Superstructure Move System

Submit shop drawings and support calculations for the proposed bridge superstructure move system.

Submit 5 copies of shop drawings and 2 copies of calculations.

The shop drawings and calculations must be sealed and signed by:

1. Engineer who is registered as a civil engineer or structural engineer
2. Independent reviewer who is:
 - 2.1. Registered as a civil engineer in the state
 - 2.2. Not employed by the entity that prepared the drawings

Include with the submittal:

1. Details and calculations for:
 - 1.1. All loads, including construction equipment loads
 - 1.2. Jacking
 - 1.3. Temporary supports
 - 1.4. Bridge superstructure move

- 1.5. Bridge superstructure move equipment to be built into or attached to the median and permanent abutments
- 1.6. Tolerances of equipment
- 1.7. Bracing for maintaining a minimum of 1/4"-clearance between the permanent abutment and bridge superstructure at all times during the bridge superstructure move
2. For equipment to be used:
 - 2.1. Description
 - 2.2. Function
 - 2.3. Limitations
 - 2.4. Locations
 - 2.5. Manufacturer's instructions
3. Stress sheets, anchor bolt layouts, shop details, and erection and removal plans
4. For the jacking system:
 - 4.1. Details and calculations for jacking and supporting the structure
 - 4.2. Lateral stiffness
 - 4.3. Redundant system of supports if the jack fails
 - 4.4. Methods of assuring synchronized, uniform jacking at all locations
 - 4.5. Means to control twist in the superstructure while jacking
5. For the bridge superstructure move:
 - 5.1. Maximum coefficient of friction between the sliding points
 - 5.2. Slide forces considered, including an accurate weight takeoff of the total weight of the new superstructure
 - 5.3. Maximum wind load acceptable for conducting bridge superstructure move operation
 - 5.4. Calculations of the bridge superstructure move equipment capacity
 - 5.5. Details and calculations for anchoring the bridge superstructure move equipment
 - 5.6. Means and methods to ensure a smooth surface for bridge superstructure move
 - 5.7. Means and methods to control movement during bridge superstructure move operation
6. Summary of computed stresses in the temporary supports, superstructure and substructure continuously during the bridge superstructure move operation. The computed stresses must include the effect of the jacking sequence and bridge superstructure move operations. Calculations must include a lateral stiffness assessment of the jacking support system.
7. Calculations for differential deflections while transferring superstructure to its permanent location
8. Means and methods for removal of bridge superstructure move system, including temporary supports, jacking system, and sliding components
9. Points where twist is monitored
10. Points where deflections are monitored

60-6.01C(3)(c) Geometric Control System

Submit 2 copies of the initial survey of the median abutment, permanent abutment, and the new superstructure conducted before beginning trial bridge superstructure move operations within 2 days of completing the survey.

Submit within 2 days of completion, 2 copies of the final survey of the median abutment, permanent abutment, and the new superstructure, conducted after the bridge superstructure move is complete.

Surveys must be sealed and signed by one of the following:

1. Land surveyor licensed in the State
2. Engineer who is registered as a civil engineer in the State

Submit proposed method of geometric control system to monitor displacement and geometric alignment of the new superstructure.

The submittal must include:

1. Details, functions and locations of the geometric control system equipment
2. Means and methods to monitor displacement and geometric alignment during the bridge superstructure move operation
3. The location and values of permanent benchmarks and control points to be used in monitoring geometric alignment during the bridge superstructure move operations

4. Details of lateral and longitudinal location control points on the new superstructure that correspond to, or can be referenced to, appropriate lateral and longitudinal control points at the permanent abutment.
5. Schedule of measurements when the new superstructure is being jacked and/or during bridge superstructure move operation
6. Methods to determine differential displacement during jacking and bridge superstructure move
7. Means and methods to meet tolerances specified herein. These tolerances must be monitored:
 - 7.1. During the actual and trial bridge superstructure moves
 - 7.2. After the move is complete but still on transporting mechanism
 - 7.3. When resting on bearing pads
 - 7.4. Immediately after the move.
8. Required elevations and tolerances with sufficient clearances to accommodate the bridge superstructure move system.
9. Process for maintaining record of observations and operations through-out the time the superstructure is in bridge superstructure move operation

60-6.01C(4) Work Plans

60-6.01C(4)(a) General

Work plans must be authorized before starting related work.

Work plans for the bridge superstructure move and trial bridge superstructure move must be authorized before the bridge superstructure move preconstruction meeting.

Allow 20 days for review of the work plans.

60-6.01C(4)(b) Bridge Superstructure Move Work Plan

Submit work plans for the trial bridge superstructure move and actual bridge superstructure move before performing a trial bridge superstructure move of the bridge.

The work plan must include:

1. Sequence of operations in the bridge superstructure move operation and the schedule of each operation to demonstrate that bridge superstructure move will be completed within the allotted time to open the bridge for traffic
2. Personnel involved in bridge superstructure move operation and division of work between personnel
3. Organizational chart showing the personnel involved and the lead person for each activity
4. Communication plan between Contractor and subcontractors to remain in contact with each other during the bridge superstructure move operation
5. Preoperational checklists for checking procedures before the bridge superstructure move starts
6. Operational details for control of movement and check-off list to assure that bridge superstructure move movement is satisfactory as the operation progresses
7. Means and methods to correct any differential forward movement/ bridge superstructure move in the bridge superstructure during the bridge superstructure move operation
8. Details of the personnel training and the specific bridge superstructure move operations being targeted for training
9. Dates and locations of training activities

If ordered, submit a revised work plan for the actual bridge superstructure move after the trial bridge superstructure move is completed. The revised work plan must be authorized before the actual bridge superstructure move. Allow 5 days for the review of the revised work plan.

The revised work plan must include:

1. Issues observed during the trial bridge superstructure move
2. Means and methods for addressing those Issues
3. Dates and locations of retraining personnel in the revisions made to the original work plan

60-6.01C(4)(c) Contingency Work Plan

Submit a contingency plan before the trial bridge superstructure move. The plan must address risk factors that may jeopardize the bridge superstructure move schedule, quality of finished product, or safety.

The plan must include risk management and traffic handling plan for:

1. Adverse weather conditions that may prohibit bridge superstructure move operation, including a warning system to notify personnel when conditions approach levels which would prohibit bridge superstructure move
2. Equipment failure before or during the bridge superstructure move operation
3. Extreme temperature immediately before or during bridge superstructure move which would affect materials or equipment to be used
4. Excessive differential displacements
5. List of critical pieces of equipment needed for bridge superstructure move
6. List of backup equipment

60-6.01C(5) Records of Observations

Submit records of observations for:

1. The trial bridge superstructure move within 24 hours of completion of trial bridge superstructure move.
2. The actual bridge superstructure move within 7 days of completion of bridge superstructure move.

60-6.01C(6) Geometric Control Logs

Submit geometric control logs for:

1. The trial bridge superstructure move within 24 hours of completion of trial bridge superstructure move.
2. The actual bridge superstructure move within 7 days of completion of bridge superstructure move.

60-6.01D Quality Assurance

60-6.01D(1) General

Calibrate each jack within 6 months of use and after each repair. Each jack and its gauge must (1) be calibrated as a unit with the cylinder extension in the approximate position that it will be at the final jacking force and (2) accompanied by a certified calibration chart. Each load cell must be calibrated. Calibration must be performed by an authorized laboratory.

60-6.01D(2) Registered Engineer

Before proceeding with trial and actual bridge superstructure move operations, the bridge superstructure move equipment, including the temporary supports and jacks must be inspected for compliance with the shop drawings by the engineer who signed the bridge superstructure move shop drawings. The engineer must certify in writing that the bridge superstructure move equipment conforms to the authorized shop drawings, and that the material and workmanship are satisfactory for the purpose intended. The certification must include any necessary testing to verify the ability of the temporary supports, jacking system, and bridge superstructure move system to sustain the stresses required by the bridge superstructure move design. A copy of this certification must always be available at the site of the work.

The licensed engineer may assign a representative to perform this certification. The assigned representative must have:

1. A license as a civil engineer
2. At least 3 years of combined experience in bridge superstructure move design and construction operations.

The Department may request you certify the experience of the assigned representative and submit supporting documentation demonstrating the required experience.

The Contractor's registered engineer must be present at the bridge site when the trial and actual bridge superstructure move operations or adjustments are in progress. The Contractor's registered engineer must continually inspect the trial and actual bridge superstructure move operations and report the records of observations in writing.

If unanticipated displacements, cracking, or other damage occur during the trial or actual bridge superstructure move operations, Contractor's registered engineer must submit a proposal immediately to the Engineer for authorization to remedy the occurrence.

Corrective measures must be authorized before use.

60-6.01D(3) Preconstruction Meeting

Schedule and hold a bridge superstructure move preconstruction meeting at least 10 days before starting the trial bridge superstructure move operation. You must provide a meeting facility.

The meeting must include the Engineer, your representatives, representatives from the bridge superstructure move Contractor, and representatives from any other subcontractor to be involved in the bridge superstructure move operation.

The Engineer conducts the meeting. Be prepared to discuss:

1. Authorized shop drawings and work plans for bridge superstructure move
2. Timelines and critical path activities
3. Contractual relationships and delineation of responsibilities among you and the subcontractors
4. Contacts and communication protocol between you and your representatives, the subcontractors, and the Engineer
5. Coordination of the construction schedule and activities
6. Structural, roadway, and construction requirements for opening the median and permanent bridges to traffic within the closure times allowed under section 12.
7. Contingency plans in case of an unforeseen event
8. Future meetings and safety requirements

60-6.01D(4) Geometric Control System

Monitor the jacking support system, the bridge superstructure, median abutment, and the permanent abutment continuously during the trial bridge superstructure move and actual bridge superstructure move operations. Use vandal-resistant geometric control system. Monitoring records must be signed by an engineer who is registered as a civil engineer or land surveyor in the State.

As a minimum, monitor the bridge superstructure at the following locations:

1. Centerline of the abutments (median and permanent)
2. Midspan
3. At each corner

Locate control points at each location near the center and at both edges of the superstructure.

As a minimum, record elevations at the following times during the trial bridge superstructure move and actual bridge superstructure move:

1. Before starting jacking activities
2. Immediately after completing jacking
3. Continuously while the structure is being moved laterally
4. Before connecting the superstructure to the substructure
5. After removing jacking support system

60-6.01D(5) Trial Bridge Superstructure Move

Trial bridge superstructure move must demonstrate that:

1. The superstructure is being jacked at all jacking points uniformly.
2. The differential displacement between jacking points does not exceed the specified tolerances during the jacking or trial bridge superstructure move operations.
3. The superstructure is sliding uniformly at both abutments without any differential forward movement of the superstructure. If differential forward slide is observed at the abutments, then the means and methods for correcting it are successful.
4. The geometric control system is providing continuous real-time monitoring of the vertical and horizontal displacements during jacking or trial bridge superstructure move operations.

5. Coordination and communication between the personnel is satisfactory throughout the trial bridge superstructure move operation
6. Data recording protocols for trial bridge superstructure move operations were followed

60-6.02 MATERIALS

60-6.02A General

Not Used

60-6.02B Sliding Material

If sliding devices are used for bridge superstructure move, use PTFE sliding material with stainless steel.

PTFE must be one of the following:

1. Lubricated Dimpled Unfilled Virgin PTFE
2. Non-lubricated Smooth Unfilled Virgin PTFE

Stainless steel must be Type 304 (ASTM A167 or A264) with mirror finish that corresponds to a surface finish variation of 0.8 μ -in RMS or better.

Stainless steel can also be #2B finish if Lubricated Dimpled Unfilled Virgin PTFE is being used.

60-6.02C Design Criteria

Jacks must be able to be controlled as a group and/or as individual units. Provide controls to reset jacks as a group and/or as individual units.

The jacking support system must resist the actual structure dead load and lateral design forces shown, plus any additional loads from bridge superstructure move system, including jacking system, and activities. As a minimum, the horizontal load to be resisted in any direction for the jacking support system and temporary bracing must be (1) the sum of actual horizontal loads due to equipment, construction sequence, or other causes plus an allowance for wind as specified in Section 48-2.02B(2) and (2) not less than 2 percent of the total dead load of the structure being jacked. Jacks must be supported on the existing abutments. If the jacking support stiffness exceeds the described minimum stiffness, increase the lateral design forces to be compatible with the jacking support lateral stiffness.

Systems involving modifications to the bridge that impair the structural integrity, intended serviceability, or design capacity of the bridge are not allowed.

60-6.03 CONSTRUCTION

60-6.03A General

During the entire time the trial or actual bridge superstructure move operations are in progress, prepare:

1. Logs of geometric control system
2. Records of observations

Twist must be monitored at least at four points (over centerlines of permanent span support bearings at girders) of the superstructure.

Relative deflections must be monitored at least at six points (over centerlines of the girders and at centerline of bridge) of the superstructure.

60-6.03B Trial Bridge Superstructure Move

Perform a trial bridge superstructure move of at least 6- inches towards the permanent abutment and back 1 week before the actual superstructure move.

The trial superstructure move must be completed within 2 hours from start to finish.

60-6.03C Backup Equipment

Before starting bridge superstructure move activities, you must:

1. Mobilize and store critical pieces of back-up equipment required to perform bridge superstructure move operations. Back-up equipment must be stored on-site.

2. Retain back-up bridge superstructure move equipment until bridge superstructure move operations have been completed.

60-6.03D Jacking

Equip each jack with a pressure gauge or load cell for determining the jacking force. Each pressure gauge must have an accurately reading dial at least 6 inches in diameter. Each load cell must be provided with an indicator to determine the jacking force.

Provide a redundant system of supports during jacking activities. The redundant system must include stacks of steel plates added as necessary to maintain the redundant supports within 1/4 inch of the jacking sill or corbels.

Before moving the superstructure, the jacking support system must (1) apply a force to the structure that is equal to the initial jacking load or the dead load shown and (2) hold that load until all initial compression and settlement of the system is completed.

The jacking locations must bear directly on center line of each abutment diaphragm. When raising and lowering the superstructure, the jacking loads must be operated simultaneously to all the jacking locations throughout the width of the superstructure.

60-6.03E Bridge Superstructure Move

Follow authorized procedures during the trial and actual bridge superstructure move operations. Any deviation from the authorized plan must be resubmitted for approval.

Implement authorized preoperational checking procedures before the bridge superstructure move operation to ensure satisfactory completion.

Monitor and carefully control the trial and actual superstructure move operations such that:

1. Specified tolerances are not exceeded at any time.
2. No unanticipated displacement, cracking, or other damage occurs

Ensure that the bridge superstructure move loads, including the jacking loads, are applied simultaneously to prevent distortion and excessive stresses that would damage the structure.

Lower the superstructure to the position shown on the plans such that the load is distributed uniformly across each abutment. Place galvanized shims as authorized by the Engineer, when required to provide uniform loading at bearing pads.

60-6.03F Tolerances During Bridge Superstructure Move

Maintain a clearance of at least 1/4-inch between the bridge superstructure and permanent abutments at all times during the bridge superstructure move. Provide bracing as required to maintain the clearance.

The bridge superstructure move speed must not exceed 10 in/min at any time.

Jack the superstructure as necessary to maintain the total vertical displacements at control points to less than 1/4-inch from the elevations recorded before jacking or as modified by the Engineer.

Twist must not exceed the lesser of $W/200$ or 0.25 feet.

Relative deflection must not exceed the lesser of $W/400$ or 0.125 feet.

W is defined as the perpendicular distance in feet between the centerlines of the external girders.

60-6.03G Tolerances For Superstructure In Its Final Position

Do not exceed 1/4- inch maximum deviation at each end of span from overall longitudinal alignment after setting.

Do not exceed 1/4-inch maximum deviation from overall transverse location (i.e., longitudinal position) at each center line of bearings.

Add to section 83-2.01A(1):

Locations shown are approximate. The Engineer will determine exact locations. Confirm layout with the Engineer prior to installation.

Add to section 83-2.01A(2):

Sand bedding must comply with section 19-3.02F(2).

Controlled low-strength material must comply with section 19-3.01C(6) and 19-3.02G, except the 28-day compressive strength must be from 100 to 120 psi.

Add to section 83-2.01A(3):

Use sand bedding or other authorized material to:

1. Backfill holes resulting from the removal of existing posts or anchors if the holes are not reused
2. Partially backfill holes so that the bottom of the post bears on compacted material if a post of the same size is being replaced
3. Completely backfill holes resulting from the removal of a wood post before installing a steel post in the same hole
4. Completely backfill holes resulting from drilling a pilot hole before driving a steel post in the hole
5. Backfill any void remaining after installing a post or sleeve
6. Backfill holes in pavement around posts, up to the bottom of controlled low-strength material as shown.

Moisten and thoroughly compact sand bedding or other authorized backfill material.

Drive steel foundation tubes with soil plates attached with or without pilot holes, or place them in drilled holes. Backfill any space around the foundation tubes with sand bedding. Place the material in 4-inch-thick layers. Moisten and thoroughly compact each layer. Coat the inside surfaces of the foundation tubes to receive wood terminal posts with grease. Insert the posts into the tubes by hand. Do not drive the posts. You may slightly round the post edges to facilitate insertion.

If a soil plate is not used on steel foundation tube or soil tube, you must install a 5/8 inch bolt through the tube just below the bottom of the post to prevent the post from dropping into the tube.

Identify steel posts longer than 6 feet by painting the length as a 2-inch tall numeral in the web near the top of the post before installation. Use black metallic acrylic resin type paint applied to a clean dry surface.

At locations exposed to traffic, schedule activities so that at the end of each day both leading and trailing ends of all guardrail or barriers are anchored to an authorized permanent or temporary terminal system, end anchor assembly or anchor block. You may anchor to existing guardrail using a standard bolted rail element splice.

Replace *Not Used* in section 83-2.01A(4) with:

Sand bedding is included in the payment for metal railings and barriers.

Additional posts, CRT posts, blocks, or hardware installed to avoid obstacles is included in the payment for metal railings and barriers.

Shop bent rail elements are included in the payment for metal railings and barriers

Replace item 1 in the list in the 2nd paragraph of section 83-2.02C(1)(a) with:

1. Steel line posts.

Replace item 2 in the list in the 2nd paragraph of section 83-2.02C(1)(a) with:

2. Plastic blocks having a nominal depth of 8 inches for steel line posts. You may use wood blocks for W6x15 steel posts.

Add to section 83-2.02C(1)(a):

The exposed bolt threads on guardrail beyond the nut that are more than 0.5 inch must be cut off.

Replace *Reserved* in section 83-2.02C(3) with:

The offset from the face of the Type WB-31 transition railing to the hinge point must be at least 3'-6".

The offset from the face of the adjacent midwest guardrail system to the hinge point must be transitioned from the offset at the Type WB-31 transition railing to 4'-0" using a ratio of 6:1.

Replace section 83-2.04A with:

83-2.04A(1) General

Section 83-2.04A includes specifications for constructing terminal systems.

83-2.04A(1)(a) Submittals

Submit a certificate of compliance for each model of terminal system installed.

At least 10 days before installation, for each model of terminal system used on the project, submit at least two copies of the following:

1. Manufacturer's:
 - 1a. Caltrans-authorized drawings
 - 1b. Instruction manuals with installation checklists
 - 1c. Maintenance manuals
2. Installation locations for each model

For each terminal system installed, submit a completed manufacturer's checklist within 10 days after installation.

83-2.04A(2) Materials

Not Used.

83-2.04A(3) Construction

Use personnel trained by the manufacturer to install terminal systems.

For each model of terminal system being installed, have a copy of the manufacturer's drawings and installation manuals onsite.

As each terminal system is installed, complete the manufacturer's installation checklist and include the following:

1. Contract number
2. Name of installation Contractor
3. Type of terminal system installed
4. Flare offset used in layout
5. Date of installation
6. Location on the project by post mile, and by station if stationing is shown
7. Name and signature of person completing the checklist

Identify each terminal system installed by painting the terminal system type in 2-inch-high, neat, black letters and figures on the backside of the rail element between system posts number 4 and 5.

Before applying paint, the rail surface must be free of all dirt, grease, oil, salt, or other contaminants by washing it with detergent or other suitable cleaner. Rinse thoroughly with fresh water and allow to fully dry.

Paint must be metallic acrylic resin type spray paint.

Install and backfill steel foundation tubes and soil tubes used in terminal systems in conformance with the manufacturer's recommendations and sections 83-2.01A(3) and 83-2.02C(1)(b).

83-2.04A(4) Payment

Not Used

Replace Reserved in section 83-2.04B with:

83-2.04B(1) General

83-2.04B(1)(a) Summary

Section 83-2.04B includes specifications for constructing alternative inline terminal systems.

83-2.04B(2) Materials

Use the following options if allowed by the manufacturer and as described:

1. Steel posts
2. Plastic blocks having a nominal depth matching the railing they will be attached to

Alternative inline terminal systems must be the following or a Department-authorized equal and must meet MASH Test Level 3 criteria as described:

1. Type Soft-Stop terminal system - Type Soft-Stop terminal system must be a Soft-Stop terminal with a system length of 50'-9-1/2" for test level 3, manufactured by Trinity Highway Products, LLC, and must include items detailed for soft-stop terminal system, as shown. The soft-stop terminal can be obtained from the manufacturer:

Address	Telephone no.
TRINITY HIGHWAY PRODUCTS LLC PO BOX 99 CENTERVILLE UT 84012	(800) 772-7976

2. Type MSKT - Type MSKT terminal system must be a 31 inch MSKT Guard Rail End Terminal with a system length of 50'-0" as manufactured by Road Systems, Inc., located in Big Spring, Texas, and must include items detailed for Type MSKT terminal system shown on the plans and must use 8 inch blocks. The MSKT Guard Rail End Terminal can be obtained from the distributors:

Address	Telephone no.
UNIVERSAL INDUSTRIAL SALES PO BOX 699 PLEASANT GROVE UT 84062	(801) 785-0505
GREGORY INDUSTRIES INC 4100 13TH ST SW CANTON OH 44708	(330) 477-4800

3. Type MAX-Tension Tangent Guardrail End Treatment by Barrier Systems, Inc. is a tangent, re-directive gating guardrail terminal. The MAX-Tension has a length of 55'-1/2", and can be flared for an offset of 0 to 2 feet at the head. The MAX-Tension terminal can be obtained from the distributor:

Replace the row for bridge deck concrete in the table in the 1st paragraph of section 90-1.02A with:

Bridge deck concrete	0.032
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Add to section 90-1.02G(6):

For concrete at 04-0313L, 04-0024R, and 04-0023R, except for the concrete at abutments and wingwalls, the ratio of the quantity of free water to the quantity of cementitious material must not exceed 0.45.

Add to section 90-1.02H:

Concrete at 04-0313L is in a corrosive environment.

For concrete at bottom slab of box girder at Bridge No. 04-0313L, the cementitious material must be composed of one of the following, by weight:

1. 20 percent natural pozzolan or fly ash with a CaO content of up to 10 percent, 5 percent silica fume, and 75 percent portland cement
2. 12 percent silica fume, metakaolin, or UFFA, and 88 percent portland cement
3. 50 percent GGBFS and 50 percent portland cement

For concrete at the wingwall and the abutment, the ratio of the quantity of free water to the quantity of cementitious material must not exceed 0.40.

Add to section 90-1.02:

90-1.02K Polymer Fibers

Fibers must comply with ASTM D7508. Microfibers must be from 1/2 to 2 inches long. Macrofibers must be from 1 to 2-1/2 inches long.

**REVISED STANDARD SPECIFICATIONS
APPLICABLE TO THE 2018 EDITION
OF THE STANDARD SPECIFICATIONS**

Add between the 3rd and 4th paragraphs of section 2-1.15C(1):

10-19-18

Submit a copy of the quote from each DVBE listed on the Certified DVBE Summary form that describes the type and dollar amount of work shown on the form no later than 4 p.m. on the 4th business day after bid opening.

Add between the 1st and 2nd paragraphs of section 2-1.18C:

10-19-18

Failure to submit a completed Certified Small Business Listing for the Non–Small Business Preference form by 4 p.m. on the 2nd business day after bid opening will result in a nonresponsive bid.

Replace section 2-1.33B with:

10-19-18

2-1.33B Bid Form Submittal Schedules

2-1.33B(1) General

The *Bid* book includes forms specific to the Contract. The deadlines for the submittal of the forms vary depending on the requirements of each Contract. Determine the requirements of the Contract and submit the forms based on the applicable schedule specified in section 2-1.33B.

Bid forms and information on the form that are due after the time of bid may be submitted at the time of bid.

2-1.33B(2) Federal-Aid Contracts

2-1.33B(2)(a) General

Section 2-1.33B(2) applies to a federal-aid contract.

2-1.33B(2)(b) Contracts with a DBE Goal

2-1.33B(2)(b)(i) General

Section 2-1.33B(2)(b) applies if a DBE goal is shown on the *Notice to Bidders*.

2-1.33B(2)(b)(ii) Bid Form Submittal

Submit the bid forms according to the schedule shown in the following table:

**Bid Form Submittal Schedule for a
Federal-Aid Contract with a DBE Goal**

Form	Submittal deadline
Bid to the Department of Transportation	Time of bid except for the public works contractor registration number
Copy of the Bid to the Department of Transportation as submitted at the time of bid with the public works contractor registration number	10 days after bid opening
Subcontractor List	Time of bid except for the public works contractor registration number
Copy of the Subcontractor List as submitted at the time of bid with the public works contractor registration number	10 days after bid opening
Small Business Status	Time of bid
Opt Out of Payment Adjustments for Price Index Fluctuations ^a	Time of bid
DBE Commitment	No later than 4 p.m. on the 5th day after bid opening ^b
DBE Confirmation	No later than 4 p.m. on the 5th day after bid opening ^b
DBE Good Faith Efforts Documentation	No later than 4 p.m. on the 5th day after bid opening ^b

^aSubmit only if you choose the option.

^bIf the last day for submitting the bid form falls on a Saturday or holiday, it may be submitted on the next business day with the same effect as if it had been submitted on the day specified.

2-1.33B(2)(b)(iii) Reserved

2-1.33B(2)(c) Contracts without a DBE Goal

2-1.33B(2)(c)(i) General

Section 2-1.33B(2)(c) applies if a DBE goal is not shown on the *Notice to Bidders*.

2-1.33B(2)(c)(ii) Bid Form Schedule

Submit the bid forms according to the schedule shown in the following table:

**Bid Form Submittal Schedule for a
Federal-Aid Contract without a DBE Goal**

Form	Submittal deadline
Bid to the Department of Transportation	Time of bid except for the public works contractor registration number
Copy of the Bid to the Department of Transportation as submitted at the time of bid with the public works contractor registration number	10 days after bid opening
Subcontractor List	Time of bid except for the public works contractor registration number
Copy of the Subcontractor List as submitted at the time of bid with the public works contractor registration numbers	10 days after bid opening
Small Business Status	Time of bid
Opt Out of Payment Adjustments for Price Index Fluctuations ^a	Time of bid

^aSubmit only if you choose the option.

2-1.33B(2)(c)(iii) Reserved

2-1.33B(2)(d)–2-1.33B(2)(h) Reserved

2-1.33B(3) Non-Federal-Aid Contracts

2-1.33B(3)(a) General

Section 2-1.33B(3) applies to non-federal-aid contracts.

2-1.33B(3)(b) Contracts with a DVBE Goal

2-1.33B(3)(b)(i) General

Section 2-1.33B(3)(b) applies if a DVBE goal is shown on the *Notice to Bidders*.

2-1.33B(3)(b)(ii) Bid Form Submittal

Submit the bid forms according to the schedule shown in the following table:

**Bid Form Submittal Schedule for a
Non-Federal-Aid Contract with a DVBE Goal**

Form	Submittal deadline
Bid to the Department of Transportation	Time of bid except for the public works contractor registration number for a joint-venture contract
For a joint-venture contract, copy of the Bid to the Department of Transportation as submitted at the time of bid with the public works contractor registration number	10 days after bid opening
Subcontractor List	Time of bid
Opt Out of Payment Adjustments for Price Index Fluctuations ^a	Time of bid
Certified DVBE Summary	No later than 4 p.m. on the 4th business day after bid opening
California Company Preference	Time of bid
Request for Small Business Preference or Non–Small Business Preference ^a	Time of bid
Certified Small Business Listing for the Non–Small Business Preference ^a	No later than 4 p.m. on the 2nd business day after bid opening

^aSubmit only if you choose the option or preference.

2-1.33B(3)(b)(iii) Reserved

2-1.33B(3)(c) Contracts without a DVBE Goal

2-1.33B(3)(c)(i) General

Section 2-1.33B(3)(c) applies if a DVBE goal is not shown on the *Notice to Bidders*.

2-1.33B(3)(c)(ii) Bid Form Submittal

Submit the bid forms according to the schedule shown in the following table:

2. Search results from the Department of General Services' website of available DVBEs
3. Communication with a DVBE community organization nearest the job site, if applicable
4. Documented communication with DVBEs describing the work to be performed, the percentage of the total bid, the corresponding dollar amount, and the responses to the communication

Replace section 5-1.24 with:

10-19-18

5-1.24 CONSTRUCTION SURVEYS

5-1.24A General

The Department places stakes and marks under chapter 12, "Construction Surveys," of the Department's *Surveys Manual*.

Submit your request for Department-furnished stakes:

1. Once staking area is ready for stakes
2. On a Request for Construction Staking form

After your submittal, the Department starts staking within 2 business days.

Preserve stakes and marks placed by the Department. If the stakes or marks are destroyed, the Department replaces them at the Department's earliest convenience and deducts the cost.

Replace section 5-1.26 with:

10-19-18

5-1.26 RESERVED

Replace item 1.2 in the list in the 1st paragraph of section 5-1.43E(2)(b) with:

10-19-18

- 1.2. Have completed training by the Department

Replace item 1.2 in the list in the 1st paragraph of section 5-1.43E(3)(b) with:

10-19-18

- 1.2. Have completed training by the Department

AA

6 CONTROL OF MATERIALS

04-19-19

Replace section 6-1.03 with:

04-19-19

6-1.03 LOCAL MATERIALS

6-1.03A General

Local material must be rock, sand, gravel, earth, or mineral material other than local borrow, or selected material obtained or produced from a source in the work vicinity, specifically for use on the project. Local borrow must not be a material from an established commercial source.

Upon your request, the Department tests material for quality characteristics from an untested local source. If satisfactory material from that source is used in the work, the Department does not charge you for the tests; otherwise, the Department deducts the test costs.

AA

7 LEGAL RELATIONS AND RESPONSIBILITY TO THE PUBLIC

04-19-19

Replace the 6th through 10th paragraphs of section 7-1.02K(3) with:

04-19-19

You may submit certified payroll records electronically using the Department’s secure file transfer protocol site. For information on electronic submission of certified payroll records, go to the Department’s Division of Construction website.

Submit payroll records electronically in a nonmodifiable PDF file, using the following file-naming convention:

TT-EA-WE-DOCTYPE.PDF

where:

TT = district, leading zero

EA = Contract number, excluding the district identification number, expressed as 6 characters

WE = week ending date entered as month, leading zero; day of month, leading zero; year, last 2 digits

DOCTYPE = labor payroll document type, CP for Certified Payroll, FB for Fringe Benefit Statement, or SC for Statement of Compliance

Before submitting the payroll records electronically, you and your subcontractors must each complete and sign the Request for Electronic Submission of Certified Payroll Records and e-mail it in PDF format to the district Labor Compliance Office. The Department provides you and your subcontractors' assigned representatives the accounts and user identifications by e-mail after each Request for Electronic Submission of Certified Payroll Records is received.

Each electronic submission must:

1. Include certified payroll records in a nonmodifiable PDF file
2. Include a signed Statement of Compliance form with each weekly record as a nonmodifiable PDF file
3. Be received by the Department by close of business on the 15th day of the month for the prior month's work

Replace the 1st sentence in the 5th paragraph of section 7-1.02K(6)(a) with:

10-19-18

Submit copies of your Injury and Illness Prevention Program, Code of Safe Practices, and permits required by Cal/OSHA as informational submittals.

Replace *Reserved* in section 7-1.02M(2) with:

04-19-19

Submit the names and emergency telephone numbers of the nearest fire suppression agencies before the start of job site activities as an informal submittal. Post the names and phone numbers at a prominent place at the job site.

Cooperate with fire prevention authorities in performance of the work.

Immediately report fires occurring within and near the project limits by dialing 911 and to the nearest fire suppression agency by using the emergency phone numbers retained at the job site.

Prevent project personnel from setting open fires that are not part of the work.

Prevent the escape of and extinguish fires caused directly or indirectly by job site activities.

Replace the 2nd paragraph of section 7-1.02M(3) with:

04-19-19

For the list of permitted sites, go to the Department of Conservation, Division of Mine Reclamation website.

AA

8 PROSECUTION AND PROGRESS

04-19-19

Replace the row for Safety in the table in the 2nd paragraph of section 8-1.03 with:

10-19-18

Safety	Injury and Illness Prevention Program, Code of Safe Practices, and job site posters
--------	---

Replace item 3 in the list in the 3rd paragraph of section 8-1.07C with:

04-19-19

- 3. Delay days exclude Saturdays and holidays.

Replace section 8-1.14E with:

04-19-19

8-1.14E Payment Adjustment for Termination

If the Department issues a termination notice, the Engineer determines the payment for termination based on the following:

- 1. Direct cost for the work performed:
 - 1.1. Including:
 - 1.1.1. Mobilization.
 - 1.1.2. Demobilization.
 - 1.1.3. Securing the job site for termination.
 - 1.1.4. Losses from the sale of materials.
 - 1.2. Not including:
 - 1.2.1. Cost of materials you keep.
 - 1.2.2. Profit realized from the sale of materials.
 - 1.2.3. Cost of material damaged by:
 - 1.2.3.1. Act of God.
 - 1.2.3.2. Act of a public enemy.
 - 1.2.3.3. Fire.
 - 1.2.3.4. Flood.
 - 1.2.3.5. Governor-declared state of emergency.
 - 1.2.3.6. Landslide.
 - 1.2.3.7. Tsunami.
 - 1.2.4. Other credits.
- 2. Cost of remedial work, as estimated by the Engineer, is not reimbursed.
- 3. Allowance for profit not to exceed 4 percent of the cost of the work performed where a likelihood of having made a profit had the Contract not been terminated is shown.
- 4. Material handling costs for material returned to the vendor or disposed of as ordered.
- 5. Costs in determining the payment adjustment due to the termination, excluding attorney fees and litigation costs.

6. Overhead costs.

Termination of the Contract does not relieve the surety of its obligation for any just claims arising out of the work performed.

AA

9 PAYMENT

04-19-19

Replace section 9-1.07B(5) with:

10-19-18

9-1.07B(5) Hot Mix Asphalt Containing Reclaimed Asphalt Pavement

The Engineer calculates the quantity of asphalt in HMA containing RAP using the following formula:

$$Qrap = HMARTT \times Xaa$$

where:

$$Xaa = Xta - [(Xrap \times Xra \times (Xta - 100)) / (100 \times (Xra - 100))]$$

and:

Qrap = quantity in tons of asphalt used in HMA containing RAP

HMARTT = HMA containing RAP, total tons placed

Xaa = asphalt content of HMA containing RAP adjusted to exclude the asphalt content in RAP, expressed as a percentage of the total weight of HMA containing RAP

Xta = total theoretical asphalt content in HMA containing RAP from the job mix formula, expressed as a percentage of the total weight of HMA containing RAP

Xrap = RAP percentage in HMA containing RAP from the job mix formula, expressed as a percentage of the total dry weight of aggregate in HMA containing RAP

Xra = average asphalt content of RAP from the job mix formula, expressed as percentage of total weight of RAP

Replace the 2nd sentence in the 7th paragraph of section 9-1.11E with:

04-19-19

The cost is determined under section 9-1.05 except no markup is allowed.

Replace section 9-1.16C with:

10-19-18

9-1.16C Materials On Hand

A material on hand but not incorporated into the work is eligible for a progress payment if:

1. Compliant with other Contract parts
2. Material cost exceeds either of the following:
 - 2.1. \$50,000
 - 2.2. \$25,000 if the requestor is certified as one or more of the following:
 - 2.2.1. DVBE
 - 2.2.2. DBE
 - 2.2.3. Small business as certified by Department of General Services, Office of Small Business and Disabled Veteran Business Enterprise Services
3. Purchased
4. Invoice is submitted
5. Stored within the State and you submit evidence that the stored material is subject to the Department's control
6. Protected from weather and contamination

Replace the introductory clause in the 1st paragraph of section 11-1.03 with:

04-19-19

Replace clause 6.1.3 of AWS D1.1, the 1st paragraph of clause 9.1.2 of AWS D1.4, and clause 6.1.2 of AWS D1.5 with:

Replace the introductory clause of the 2nd paragraph of section 11-1.04 with:

04-19-19

Replace clause 6.14.6.1 of AWS D1.1, clause 9.8.1 of AWS D1.4, and clause 6.1.3.4 of AWS D1.5 with:

Add before the 1st paragraph of section 11-1.05:

04-19-19

Replace the first sentence of clause 5.21.1.1 of AWS D1.1 with the following:

5.21.1.1. The separation between surfaces of plug and slot welds, and of joints landing on a backing, shall not exceed 1/16 in [2 mm].

Replace clause 3.3.1.1 of AWS D1.5 with the following:

3.3.1.1. The separation between surfaces of plug and slot welds, and of joints landing on a backing, shall not exceed 2 mm [1/16 in].

Replace item 2 in the list in the 2nd paragraph of section 11-1.05 with:

04-19-19

2. Be mechanically and radiographically tested. Mechanical and radiographic testing and acceptance criteria must comply with the applicable AWS codes. The type of mechanical testing must be authorized.

Replace the 1st paragraph of 11-1.06 with:

04-19-19

Replace item 3 of clause 6.26.3.2 of AWS D1.5 with:

3. If indications that exhibit these planar characteristics are present at scanning sensitivity, or other evidence exists to suggest the presence of transverse cracks, a more detailed evaluation of the discontinuity by other means must be performed (e.g., alternate UT techniques, RT, grinding, or gouging for visual inspection or MT of the excavated areas.)

Replace the scanning angle in clause 6.24.2.2 of AWS D1.5 with:

Up to 45 degrees

Replace the 2nd paragraph of section 11-1.06 with:

04-19-19

Clause 6.6.5 of AWS D1.1, clause 9.6.5 of AWS D1.4, and clause 6.6.5 of AWS D1.5 do not apply.

Replace the introductory clause of the 1st paragraph of section 11-2.04 with:

04-19-19

Clauses 6.1.4.1 and 6.1.4.3 of AWS D1.1, the 2nd paragraph of clause 9.1.2 of AWS D1.4, clauses 6.1.3.1 through 6.1.3.3 of AWS D1.5, and clause 7.2.3 of AWS D1.8 are replaced with:

Delete item 2.6.3 in the list of section 13-1.01D(4)(c).

Replace the 1st paragraph of section 13-2.01C with:

04-19-19

Within 7 days after Contract approval, submit one printed copy and an electronic copy on a read-only CD, DVD, or other authorized data-storage device of your WPCP unless different quantities are ordered at the preconstruction conference. You may assign a QSP other than the WPC manager to develop the WPCP.

Replace item 4 in the list in the 2nd paragraph of section 13-2.01C with:

04-19-19

4. Show the locations and types of temporary WPC practices that will be used in the work for whichever has the longest duration in the first:
 - 4.1. 60 days
 - 4.2. Construction phase

Replace the 4th paragraph of section 13-2.01C with:

04-19-19

After the Engineer authorizes the WPCP, submit one printed copy and an electronic copy on a read-only CD, DVD, or other Engineer-authorized data-storage device of the authorized WPCP.

Delete the row for Annual Certification in the table in section 13-3.01C(1).

04-19-19

Replace the 1st paragraph of section 13-3.01C(2)(a) with:

04-19-19

Within 15 days of Contract approval, submit one printed copy and an electronic copy on a read-only CD, DVD, or other authorized data-storage device of your SWPPP unless different quantities are ordered at the preconstruction conference. You may assign a QSD other than the WPC manager to develop the SWPPP.

Replace item 4 in the list in the 2nd paragraph of section 13-3.01C(2)(a) with:

04-19-19

4. Include a schedule showing when:
 - 4.1. Work activities that could cause the discharge of pollutants into stormwater will be performed
 - 4.2. WPC practices, including soil stabilization and sediment control, that will be used in the work for whichever has the longest duration in the first:
 - 4.2.1. 60 days
 - 4.2.2. Construction phase

Replace the 4th paragraph of section 13-3.01C(2)(a) with:

04-19-19

Submit an electronic copy on a read-only CD, DVD, or other Engineer-authorized data-storage device and 4 printed copies of the authorized SWPPP unless fewer quantities are authorized at the preconstruction conference.

Replace the introductory clause in the 7th paragraph of section 13-3.01C(2)(a) with:

Submit a revised SWPPP annually before September 15th and any time:

Add after the 7th paragraph of section 13-3.01C(2)(a):

Revise the SWPPP through amendment. The annual SWPPP amendment must include an annual winterization plan.

The annual winterization plan must describe the preparation for the upcoming rainy season including:

- 1. Updated schedule
- 2. Materials and labor
- 3. Management of stormwater through the job site including:
 - 3.1. Run-on
 - 3.2. Run-off
 - 3.3. Conveyance downslope
- 4. Management of areas within the job site including:
 - 4.1. Areas where work is suspended
 - 4.2. Areas of soil stabilization
 - 4.3. New disturbed soil areas
- 5. Changes to monitoring locations
- 6. Slope stabilization

Delete section 13-3.01C(5).

AA

14 ENVIRONMENTAL STEWARDSHIP

Add between the 3rd and 4th paragraphs of section 14-10.01:

If ordered, remove solid waste from illegal dumping on the project site. This work is change order work. Illegal dumping is:

- 1. Third party nonhazardous residential or commercial waste
- 2. Greater than 1.0 cubic yard per event

Add to the beginning of section 14-11.14D:

Store treated wood waste at the jobsite until transport to the CA permitted disposal site.

Add to the beginning of section 14-11.14E:

Transport treated wood waste directly to the CA permitted disposal site after leaving the jobsite. Do not mix treated wood waste from the job site with waste from any other generator.

AA

DIVISION III EARTHWORK AND LANDSCAPE

19 EARTHWORK

10-19-18

Replace the 1st paragraph of section 19-3.03E(1) with:

10-19-18

Place structure backfill in uniform layers. Bring backfill up uniformly on all sides of structures or drainage facilities. Backfill layer thickness must not exceed 0.67 foot before compacting. If you perform compaction by ponding and jetting, the thickness of the backfill layer must not exceed 4 feet.

Replace the 1st sentence in the 3rd paragraph of section 19-3.03E(1) with:

10-19-18

Do not place structure backfill until footings or other parts of structures or drainage facilities are authorized.

AA

20 LANDSCAPE

04-19-19

Replace the 2nd paragraph of section 20-2.01A(4)(d) with:

10-19-18

In the presence of the Engineer, perform a functional test for each system that demonstrates:

1. Components of the system are functioning and integrated with one another.
2. Controller programming is complete including external weather and other system data inputs that are required to operate the system in automatic mode.
3. Watering schedule is appropriate for the plants, current weather, season, and site conditions.
4. System has complete sprinkler coverage of the site.

Perform the test for each system:

1. Before planting the plants
2. After irrigation system repair work
3. Annually during plant establishment work
4. Not more than 30 days prior to contract acceptance
5. When ordered

10-19-18

Delete section 20-2.01A(4)(e).

Replace the 1st paragraph of section 20-2.01B(5) with:

10-19-18

Pull boxes must comply with section 86-1.02C and be no. 5 or larger. Pull boxes for low voltage conductors must not have side openings.

Replace the 2nd paragraph of section 20-2.01B(5) with:

04-19-19

Pull box covers used for control and neutral conductors for irrigation equipment operated by the irrigation controller must be marked *SPRINKLER CONTROL*.

Add to section 20-2.01B:

04-19-19

20-2.01B(9) Woven Wire Cloth and Gravel

Woven wire cloth must be galvanized and manufactured with a minimum diameter of 19-gauge wire and have square openings from 1/4 to 1/2 inches.

Gravel must be 3/4-inch gravel or crushed rock. Gravel or crushed rock must be clean, washed, dry, and free from clay or organic material.

Replace the 1st paragraph of section 20-2.01C(2) with:

10-19-18

Perform trenching and backfilling under section 87-1.03E(2).

Replace the introductory clause to the list in the 1st paragraph of section 20-2.01C(3) with:

10-19-18

Install pull boxes under section 87-1.03C at the following locations:

Add to section 20-2.01C(4):

04-19-19

Install valve boxes on woven wire cloth and gravel or crushed rock.

Replace the 1st paragraph of section 20-2.04A(4) with:

10-19-18

Perform field tests on control and neutral conductors. Field tests must comply with the specifications in section 87-1.01D(2)(a).

Replace the 1st and 2nd paragraphs of section 20-2.04B with:

10-19-18

Control and neutral conductors must comply with the provisions for conductors and cables in section 86-1.02F.

Electrical conduit and fittings must comply with section 86-1.02(B).

Replace the 1st paragraph of section 20-2.04C(4) with:

04-19-19

Splice conductors with a UL-listed connector manufactured for copper wire, direct burial irrigation systems. Connector must be prefilled with a moisture sealing compound that encapsulates and protects the splice in a waterproof housing. Connector must be sized for the number and gauge of the conductors at the splice.

Replace the introductory clause of the 1st paragraph of section 20-2.06B(3) with:

10-19-18

The irrigation controller enclosure cabinet must comply with section 86-1.02Q and must:

Add to the beginning of section 20-2.06C:

10-19-18

Install the irrigation controller enclosure cabinet under 87-1.03Q(1).

Replace the 3rd paragraph of section 20-2.09B(1) with:

04-19-19

Threaded nipples for swing joints and risers must be schedule 80, PVC 1120 or PVC 1220 pipe, and comply with ASTM D1785.

Replace the table in the 3rd paragraph of section 20-3.01B(2)(a) with:

10-19-18

Plant group designation	Description	Container size (cu in)
A	No. 1 container	152–251
B	No. 5 container	785–1242
C	Balled and burlapped	--
E	Bulb	--
F	In flats	--
H	Cutting	--
I	Pot	--
K	24-inch box	5775–6861
M	Liner ^a	--
O	Acorn	--
P	Plugs ^{a, b}	--
S	Seedling ^c	--
U	No. 15 container	2768–3696
Z	Palm Tree	--

^aDo not use containers made of biodegradable material.

^bGrown in individual container cells.

^cBare root.

Replace the introductory clause of the 1st paragraph of section 20-3.01B(4)(b) with:

10-19-18

Slow-release fertilizer must be a pelleted or granular form with a nutrient release over a 3 to 4 month period and be within the chemical analysis ranges shown in the following table:

Replace section 20-3.01C(3) with:

10-19-18

Water plants as needed to keep the plants in a healthy growing condition.

Replace the 1st paragraph of section 20-4.03G with:

10-19-18

Operate the electric automatic irrigation systems, including external weather and other system data inputs required to operate the system in automatic mode, unless otherwise authorized.

Delete the 3rd paragraph of section 20-4.03G.

10-19-18

Replace the row for *Moisture susceptibility (min, psi, dry strength)* in the table in item 3 in the list in the paragraph of section 39-2.02A(4)(e) with:

04-19-19

For RAP substitution equal to or less than 15% moisture susceptibility (min, psi, dry strength)	AASHTO T 283	100
For RAP substitution greater than 15% moisture susceptibility (psi, dry strength)	AASHTO T 283	100-300 ^h

Add a footnote to the table in item 3 in the list in the paragraph of section 39-2.02A(4)(e):

04-19-19

^hNot required in the following areas:

1. Southern San Luis Obispo or Santa Barbara County in District 5.
2. Kern County in District 6.
3. Kings County in District 6: route 5, post mile 0 to 17; route 33, post mile 0 to 19; route 41, post mile 0 to 16.
4. Tulare County in District 6: route 65, post mile 0 to 10; route 99, post mile 0 to 10; route 43, post mile 0 to 15.

Replace the row for *Moisture susceptibility, dry strength* in the table in the 1st paragraph of section 39-2.02B(2) with:

04-19-19

For RAP substitution equal to or less than 15% moisture susceptibility (min, psi, dry strength)	AASHTO T 283	100
For RAP substitution greater than 15% moisture susceptibility (psi, dry strength)	AASHTO T 283	100-300 ^e

Add a footnote to the table in the 1st paragraph of section 39-2.02B(2):

04-19-19

^eNot required in the following areas:

1. Southern San Luis Obispo or Santa Barbara County in District 5.
2. Kern County in District 6.
3. Kings County in District 6: route 5, post mile 0 to 17; route 33, post mile 0 to 19; route 41, post mile 0 to 16.
4. Tulare County in District 6: route 65, post mile 0 to 10; route 99, post mile 0 to 10; route 43, post mile 0 to 15.

Replace the 3rd and 4th paragraphs of section 39-2.02B(2) with:

04-19-19

For RAP substitution of 15 percent or less, the grade of the virgin binder must be the specified grade of asphalt binder for Type A HMA.

For RAP substitution greater than 15 percent and not exceeding 25 percent, the grade of the virgin binder must be the specified grade of asphalt binder for Type A HMA with the upper and lower temperature classification reduced by 6 degrees C. Hamburg wheel track requirements are based on the grade of asphalt binder specified for Type A HMA.

Replace the 2nd paragraph of section 46-1.01C(3) with:

10-19-18

Submit the test data in electronic and hard copy format within 1 business day after testing is complete. Upon completion of the wall, send an email of the soil nail test results as a tabulated spreadsheet to the Engineer and Geotechnical.Data@dot.ca.gov. Include the contract number and Department's structure number of the wall in the subject line of the email.

Replace *Not Used* in section 46-1.01D(1) with:

10-19-18

Welding must comply with AWS D1.1.

Add to the end of section 46-1.03A:

10-19-18

Shotcrete must comply with section 53-2.

Delete the 3rd paragraph of section 46-1.03B.

10-19-18

Replace the 1st sentence in the 2nd paragraph of section 46-2.02B with:

10-19-18

The anchorage enclosure and the steel tube and bearing plate of the anchorage assembly must be galvanized steel and comply with sections 55-1.02D(1) and 55-1.02E(1).

Replace item 9 in the list in the 3rd paragraph of section 46-2.02D with:

10-19-18

9. Have the physical properties shown in Table 4.1 of *Recommendations for Prestressed Rock and Soil Anchors* published by the Post-Tensioning Institute

Replace the 4th paragraph of section 46-2.03D with:

10-19-18

Immediately after lock-off, perform a lift-off test to verify that the lock-off load has been attained. The lift-off load must be within 10 percent of the specified lock-off load. If necessary adjust the shim thickness to achieve the lock-off load. If the load is not within 10 percent of the specified lock-off load, the anchorage must be reset and another lift-off load reading must be made. Repeat the process until the specified lock-off load is obtained.

Replace the 2nd paragraph of section 46-3.01A with:

10-19-18

A soil nail consists of a solid steel bar with an anchorage assembly that is placed in a drilled hole and then grouted.

Replace section 46-3.01D(2)(b)(ii)(1) with:

10-19-18

46-3.01D(2)(b)(ii)(1) General

Determine the test load using the following equation:

$$T = Lb \times Qb$$

where:

T = test load, pounds

Lb = soil nail bonded length, feet, 10 feet minimum

Qb = test load per unit length of bond, pounds/foot

Replace the 8th paragraph of section 46-3.01D(2)(b)(ii)(2) with:

04-19-19

If the Engineer revises soil nail lengths or test load per unit length of bond values, any additional verification test soil nails are change order work.

Replace section 46-3.02A with:

04-19-19

46-3.02A General

Each production soil nail must be either a solid steel bar encapsulated full length in a grouted corrugated plastic sheathing or an epoxy-coated prefabricated solid steel bar partially encapsulated in a grouted corrugated plastic sheathing as shown.

Epoxy-coated prefabricated solid steel bars must comply with the specifications for epoxy-coated prefabricated reinforcement in section 52-2.03, except the average coating thickness after curing must be from 10 to 15 mils.

Solid steel bar for test soil nails is not required to be epoxy coated or encapsulated in grouted plastic sheathing.

Replace the heading of section 46-3.02B with:

10-19-18

Anchorage Assemblies

Replace section 46-3.02C with:

10-19-18

46-3.02C Solid Steel Bars

Solid steel bars must be either:

1. Threaded bars with spirally-deformed, ribbed threads continuous along the entire length of the bar.
2. Deformed reinforcing bars with at least a 6-inch length of thread cut into the bar on the anchorage end. Use coarse threading and the next larger reinforcing bar size.

Solid steel bars must comply with ASTM A615/A615M or A706/A706M, Grade 60 or ASTM A615/A615M, Grade 75.

Splicing must be authorized.

Epoxy coating at the anchorage end of epoxy-coated bars may be omitted for a maximum of 6 inches. Metal surfaces of assembled splices of epoxy-coated bars must be epoxy coated.

Choose the solid steel bar size and grade for test soil nails. Test soil nail bars must not be smaller than the production soil nails they represent.

Replace the 4th paragraph of section 48-2.02B(2) with:

10-19-18

The assumed horizontal load the falsework bracing system must resist must be the sum of the actual horizontal loads due to equipment, construction sequence or other causes, and a wind loading. The assumed horizontal load in any direction must be at least 2 percent of the total dead load.

Replace the table in the 2nd paragraph of section 48-2.02B(3)(b) with:

10-19-18

Quality characteristic	Requirement
Compression perpendicular to the grain (psi)	450
Compression parallel to the grain (psi)	$480,000/(L/d)^2$; 1,600 maximum
Flexural stress	1,800 psi; 1,500 psi maximum for members with a nominal depth of 8 inches or less.
Horizontal shear (psi)	140
Axial tension (psi)	1,200
Deflection due to concrete loading only	1/240 of span length
Modulus of elasticity (E) (psi)	1.6×10^6
Timber piles (tons)	45

NOTES:

L = unsupported length, inches

d = least dimension of a square or rectangular column or the width of a square of equivalent cross-sectional area for round columns, inches

Replace the table in the 3rd paragraph of section 48-2.02B(3)(c) with:

10-19-18

Quality characteristic	Requirement
Compression, flexural (psi)	$12,000,000/[(L \times d)/(b \times t)]^a$
Deflection due to concrete loading only	1/240 of the span
Modulus of elasticity (E) (psi)	30×10^6

NOTES:

L = unsupported length, inches

d = least dimension of rectangular columns or the width of a square of equivalent cross-sectional area for round columns, or the depth of beams, inches

b = width of the compression flange, inches

t = thickness of the compression flange, inches

F_y = specified minimum yield stress in psi

^aNot to exceed (1) 22,000 psi for unidentified steel, (2) 22,000 psi for steel complying with ASTM A36/A36M, or (3) $0.6F_y$ for other identified steel

Add to section 48-2.02:

10-19-18

48-2.02C Falsework Lighting

48-2.02C(1) General

Reserved

48-2.02C(2) Pavement Illumination

Pavement illumination fixture must:

1. Have commercial-type flood lamp holder with protective covers.
2. Be fully adjustable with brackets and locking screws.
3. Mount directly to a standard metal junction box.

4. Have a medium-base PAR-38 quartz-halogen flood lamp or an equivalent energy efficient alternative emitting 1,700 to 2,200 lumens with a correlated color temperature of 3,000 kelvin or less.

48-2.02C(3) Portal Illumination

Portal illumination includes plywood sheet clearance guides 4 feet wide by 8 feet high and fixtures with a PAR reflector floodlamp or equivalent energy efficient alternatives emitting 1,500 to 1,700 lumens with a correlated color temperature of 3,000 kelvin or less.

48-2.02C(4) Pedestrian Walkway Illumination

Pedestrian walkway illumination fixtures must be the flush mounted type equipped with a damage-resistant, clear, polycarbonate diffuser lens, an overhead protection shield, and a standard incandescent lamp or equivalent energy efficient alternatives emitting 1,500 to 2,000 lumens with a correlated color temperature of 3,000 kelvin or less.

Add to section 48-2.03A:

10-19-18

Traffic must be detoured, from the lanes over which falsework is being erected, released, or removed.

Replace the 3rd paragraph of section 48-2.03B with:

10-19-18

Falsework piles must be driven and assessed under section 49. The actual nominal pile resistance must be at least twice the falsework pile design load. For pile acceptance, the required number of hammer blows in the last foot of driving is determined using the formula in 49-2.01A(4)(c).

Add between the 2nd and 3rd paragraphs of section 48-2.03C:

10-19-18

Falsework erection includes adjustments or removal of components that contribute to the horizontal stability of the falsework system.

Replace section 48-2.03D with:

10-19-18

48-2.03D Removal

Remove falsework such that portions of falsework not yet removed remain stable at all times.

Falsework release includes blowing sand from sand jacks, turning screws on screw jacks, and removing wedges.

Except for concrete above the deck, do not release falsework supporting any span of a:

1. Simple span bridge before 10 days after the last concrete has been placed
2. Continuous or rigid frame bridge before 10 days after the last concrete has been placed:
 - 2.1. In that span
 - 2.2. In adjacent portions of each adjoining span for a length equal to one-half of the span where falsework is to be released
3. Simple span, continuous, or rigid frame bridge until the supported concrete has attained a compressive strength of 2,880 psi or 80 percent of the specified strength, whichever is greater

Do not release falsework for prestressed portions of structures until prestressing steel has been tensioned.

Do not release falsework supporting any span of a continuous or rigid frame bridge until all required prestressing is complete (1) in that span and (2) in adjacent portions of each adjoining span for a length equal to at least one half of the span where falsework is to be released.

Release falsework supporting spans of CIP girders, slab bridges, or culverts before constructing or installing railings or barriers on the spans unless authorized.

Release falsework for arch bridges uniformly and gradually. Start at the crown and work toward the springing. Release falsework for adjacent arch spans concurrently.

Do not release falsework that supports overhangs, deck slabs between girders, or girder stems that slope 45 degrees or more from vertical before 7 days after deck concrete has been placed.

You may release falsework supporting the sides of girder stems that slope less than 45 degrees from vertical before placing deck concrete if you install lateral supports. Lateral supports must be:

1. Designed to resist rotational forces on the girder stem, including forces due to concrete deck placement
2. Installed immediately after each form panel is removed
3. Installed before releasing supports for the adjacent form panel

Do not release falsework for bent caps supporting steel or PC concrete girders before 7 days after placing bent cap concrete.

Release falsework for structural members subject to bending as specified for simple span bridges.

Do not release falsework for box culverts and other structures with decks lower than the roadway pavement and span lengths of 14 feet or less until the last placed concrete has attained a compressive strength of 1,600 psi. Curing of the concrete must not be interrupted. Falsework release for other box culverts must comply with the specifications for the release of bridge falsework.

Do not release falsework for arch culverts sooner than 40 hours after concrete has been placed.

Remove falsework piling to at least 2 feet below the original ground or streambed. Remove falsework piling driven within ditch or channel excavation limits to at least 2 feet below the bottom and side slopes of the excavated areas.

Dispose of falsework materials and work debris.

Falsework removal systems employing methods of holding falsework by winches, hydraulic jacks with prestressing steel, HS rods, or cranes must also be supported by an independent support system when the falsework removal system is not actively lowering the falsework at vehicular, pedestrian, or railroad traffic openings.

Bridge deck openings used to facilitate falsework removal activities must be formed with a 6-inch maximum diameter opening. The opening must be located away from the wheel paths.

Clean and roughen openings made in the bridge deck. Fill the deck openings with rapid setting concrete complying with section 60-3.02B(2).

Bridge soffit openings used to facilitate falsework removal activities must be formed with a 5-inch maximum diameter.

Anchor 10-inch-square aluminum or galvanized steel wire, 1/4-inch-mesh hardware cloth with a 0.025-inch minimum wire diameter firmly to the inside of the soffit openings. Construct a 1/2-inch drip groove to the outside of soffit openings.

Falsework removal over roadways with a vertical traffic opening of less than 20 feet must start within 14 days after the falsework is eligible to be released and must be completed within 45 days after it is eligible to be released.

Replace section 48-2.03E with:

10-19-18

48-2.03E Falsework Lighting

48-2.03E(1) General

Provide lighting to illuminate the pavement, portals, and pedestrian walkways at or under openings in the falsework required for traffic.

Install lighting for pedestrian walkway illumination at all pedestrian openings through or under the falsework.

Design falsework lighting such that required maintenance can be performed with a minimum of inconvenience to traffic. Closing of traffic lanes for routine maintenance is not allowed on roadways with posted speed limits greater than 25 mph.

During the hours of darkness, illuminate:

1. Falsework portals
2. Pavement under falsework with portals less than 150 feet apart

Use photoelectric switches to control falsework lighting systems. Pavement under falsework with portals 150 feet or more apart and all pedestrian openings through falsework must be illuminated 24 hours per day.

Aim the lighting fixtures to avoid glare to motorists.

Fasten a Type NMC cable with no. 12 minimum conductors with ground wire to the supporting structure at sufficient intervals to adequately support the cable and within 12 inches from every box or fitting. Use 1/2-inch or larger Type 1 conduit for conductors within 8 feet of ground.

Provide a maximum 20 A fuse for each branch circuit for illumination systems at each bridge location.

Arrange with the service utility to complete service connections for falsework lighting. You pay for energy, line extension, service, and service hookup costs.

48-2.03E(2) Pavement Illumination

Install a continuous row of fixtures beneath falsework structure with the end fixtures not further than 10 feet inside portal faces. Energize the fixtures immediately after the members supporting them have been erected.

Place the fixtures along the sides of the opening not more than 4 feet behind or 2 feet in front of the roadway face of the temporary railing. Mount the fixtures from 12 to 16 feet above the roadway surface without obstructing the light pattern on the pavement.

48-2.03E(3) Portal Illumination

Provide falsework portal illumination on the side facing traffic. Mount fixtures on the structure directly over each vertical support adjacent to the traveled way, as needed, to uniformly illuminate the exterior falsework beam, the clearance guides, and the overhead clearance sign. Each fixture must be supported approximately 16 feet above the pavement and 6 feet in front of the portal face.

Portal illumination clearance guides must:

1. Be fastened vertically, facing traffic, with the bottom of the panel from 3 to 4 feet above the roadway
2. Have the center of the panel located approximately 3 feet horizontally behind the roadway face of the railing
3. Be freshly painted panels for each installation with not less than 2 applications of flat white paint.

Paint testing of painted panels not required.

Portal lighting and clearance guides must be installed on the day the vertical members are erected.

If ordered, repaint the designated areas to improve the general appearance of the painted surfaces. Repainting is change order work.

48-2.03E(4) Pedestrian Walkway Illumination

Provide pedestrian walkway illumination immediately after the overhead protection shield is erected.

Flush mount the fixtures in the overhead protection shield and center them over the passageway at intervals of not more than 15 feet with the end fixtures not more than 7 feet inside the end of the pedestrian openings.

10-19-18

Delete the 4th paragraph of section 48-3.01C(2).

Add between the 9th and 10th paragraphs of section 48-3.02B:

10-19-18

For bridge removal, the temporary support system must resist the design loads and forces shown. As a minimum, the horizontal load to be resisted in any direction for temporary support shoring and temporary bracing must be (1) the sum of actual horizontal loads due to equipment, construction sequence, or other causes plus an allowance for wind and (2) not less than 5 percent of the total dead load of the structure being removed.

10-19-18

Delete the 2nd and 3rd paragraphs of section 48-4.01A.

Replace section 48-4.01C with:

10-19-18

48-4.01C Submittals

Submit shop drawings for temporary decking. Include the following:

1. Description, location, and value of all loads if temporary decking is not shown
2. Details of the connection between the temporary decking and the existing or new structure if temporary decking is not shown
3. Storage location of equipment and materials that allows for 1 shift of work and placement of temporary decking within the time allowed
4. Construction sequence and schedule details
5. Cure time for concrete to be placed under a steel plate system
6. Details for removing temporary decking and restoring the existing structure

If temporary decking is not shown, shop drawings must be signed by an engineer who is registered as a civil engineer in the State.

Replace section 48-4.01D with:

10-19-18

48-4.01D Quality Assurance

If temporary decking is not shown, the temporary decking design must comply with:

1. The unfactored permit loads, braking force, and HL93 loads except lane load from *AASHTO LRFD Bridge Specifications with California Amendments*.
2. Section 48-2.02B(3)
3. Live load deflection must not exceed 1/300 of the temporary decking span for the design load.
4. Temporary decking must have a uniform surface with a coefficient of friction of at least 0.35 when measured under California Test 342.
5. Steel plate systems must be mechanically connected to the existing structure and adjacent approaches. If a steel plate spans a joint, the mechanical connection must accommodate at least 50 percent of the movement rating shown for that joint.

6. Must not overstress, induce permanent forces into, or produce cracking in the existing structure.

Replace section 48-4.03 with:

10-19-18

48-4.03 CONSTRUCTION

Temporary decking must consist of one of the following:

1. Steel plate system that spans the incomplete work.
2. Falsework with an asphalt concrete surface that spans the incomplete work. Do not use falsework with an asphalt concrete surface to cover deck concrete that has not cured or to cover partially installed joint materials.

Construct temporary decking under the specifications for falsework in section 48-2 except the first paragraph of section 48-2.03D does not apply.

If there is an elevation difference of more than 1/2 inch between the temporary decking and the adjacent deck, install temporary tapers up to and away from the temporary decking. Construct tapers under section 7-1.03. If the temporary decking does not extend the entire width of the roadway, taper the sides of the temporary decking at a 12:1 (horizontal: vertical) ratio.

Material for temporary tapers must comply with section 60-3.02B(2) or 60-3.04B(2). Cure temporary tapers at least 3 hours before allowing traffic on the temporary decking.

If unanticipated displacements, cracking, or other damage occurs to the existing structure or to any new components installed in or adjacent to the deck, stop work on the deck and perform corrective measures.

Edges of steel plate systems must be in full contact with the existing deck and the adjacent approach slab. If used, shims must be securely attached to the plate.

For falsework with an asphalt concrete cover, asphalt concrete must be at least 3 inches thick and compacted in place.

Do not allow traffic on deck concrete until it has attained the design compressive strength shown.

When temporary decking is no longer needed, remove temporary decking materials and connections from the existing structure as soon as possible. Remove modifications to the existing structure except where permanent alterations are shown.

10-19-18

Delete the 4th paragraph of section 48-5.01C.

Replace the 1st paragraph of section 48-5.02B with:

10-19-18

The jacking support system must resist the structure dead load and lateral design forces shown, plus any additional loads from jacking equipment and activities. As a minimum, the horizontal load to be resisted in any direction for the jacking support system and temporary bracing must be (1) the sum of actual horizontal loads due to equipment, construction sequence, or other causes plus an allowance for wind as specified in Section 48-2.02B(2) and (2) not less than 2 percent of the total dead load of the structure being jacked. You must determine soil bearing values for support footings. If the jacking support stiffness exceeds the described minimum stiffness, increase the lateral design forces to be compatible with the jacking support lateral stiffness.

Replace the 1st paragraph of section 48-5.03 with:

10-19-18

Construct the jacking support system under the specifications for falsework in section 48-2.03.

AA

49 PILING

04-19-19

Replace the 6th paragraph of section 49-1.01D(4) with:

10-19-18

Except for load test piles and anchor piles, drive the 1st production pile in the control zone. Do not install any additional production piles until dynamic monitoring has been performed, and the Engineer provides you with the bearing acceptance criteria curves for any piles represented by the dynamically monitored piles.

Replace the 3rd paragraph of section 49-2.01D with:

10-19-18

The payment quantity for furnish piling is the length measured along the longest side of the pile from the specified tip elevation shown to the plane of pile cutoff, except for dynamically monitored piles. For dynamically monitored piles, the payment quantity for furnish piling includes an additional length of 2 times the largest cross-sectional dimension of the pile plus 2 feet.

Add to the end of section 49-2.02A(2):

10-19-18

longitudinal weld length: The length of a continuous longitudinal weld.

circumferential weld length: The length of a continuous weld around the circumference of the pipe pile.

spiral weld length: The length of one full 360-degree spiral weld revolution around the circumference of the pipe pile.

Replace the 3rd paragraph of section 49-2.02A(4)(b)(iii)(B) with:

10-19-18

For welding performed under AWS D1.1:

1. Perform NDT on 25 percent of each longitudinal, circumferential, or spiral weld length using RT or UT.
2. If repairs are required in a portion of the tested weld:
 - 2.1. Perform additional NDT on untested areas on each end of the initial portion tested. The length of additional NDT on each end must equal 10 percent of the weld length. If it is not possible to perform 10 percent of the weld length on one end, perform the remaining percentage on the other end.
 - 2.2. After this additional 20 percent of NDT is performed, determine and record the total cumulative repair lengths from all NDT for each weld length. If the cumulative weld repair length is equal to or more than 10 percent of the weld length, then perform NDT on the entire weld length.
 - 2.3. Perform NDT on the repaired portion plus 2 inches on each end of the repaired weld excavation.

Replace the 2nd paragraph of section 49-2.02A(4)(b)(iii)(C) with:

10-19-18

Perform NDT on 25 percent of the weld length performed by each welder, using RT or UT at locations selected by the Engineer. The Engineer may select several locations on a given splice. The cover pass must be ground smooth at locations to be tested.

Replace the 4th paragraph of section 49-2.02A(4)(b)(iii)(C) with:

10-19-18

If repairs are required in a portion of the tested weld:

1. Perform additional NDT on untested areas on each end of the initial portion tested. The length of additional NDT on each end must equal 10 percent of the pipe's outside circumference. If it is not possible to perform 10 percent of the weld length on one end, perform the remaining percentage on the other end.
2. After this additional 20 percent of NDT is performed, determine and record the total cumulative repair lengths from all NDT for each weld length. If the cumulative weld repair length is equal to or more than 10 percent of the pipe's outside circumference, then perform NDT on the entire weld length.
3. Perform NDT on the repaired portion plus 2 inches on each end of the repaired weld excavation.

Replace the 5th paragraph of section 49-2.02B(1)(b) with:

04-19-19

If splicing steel pipe piles using a circumferential weld, the piles must comply with the fit-up requirements of clause 9.24.1 of AWS D1.1.

Replace section 49-3.01B(2) with:

04-19-19

49-3.01B(2) Mass Concrete

Section 49-3.01B(2) applies to CIP concrete piles with a diameter greater than 8 feet.

For piles with a diameter greater than 8 feet and less than or equal to 14 feet:

1. The specifications for SCM content in the 4th paragraph of section 90-1.02B(3) do not apply.
2. The SCM content of the concrete must comply with the following:
 - 2.1. Any combination of portland cement and fly ash satisfying:

Equation 1:

$$(12 \times FM)/MC \geq X$$

where:

FM = fly ash complying with AASHTO M 295, Class F, with a CaO content of up to 10 percent, including the quantity in blended cement, lb/cu yd

MC = minimum quantity of cementitious material specified, lb/cu yd

X = 3.0 for $8 < D \leq 10$, where *D* = pile diameter in feet

X = 4.0 for $10 < D \leq 14$, where *D* = pile diameter in feet

Equation 2:

$$MC - MSCM - PC \geq 0$$

where:

MC = minimum quantity of cementitious material specified, lb/cu yd

MSCM = minimum sum of SCMs that satisfies equation 1, lb/cu yd

PC = quantity of portland cement, including the quantity in blended cement, lb/cu yd

- 2.2. You may replace any portion of the portland cement with any SCM complying with section 90-1.02B(3) if equations 1 and 2 are satisfied as specified above.

For piles with a diameter greater than 14 feet, the concrete must comply with the specifications for mass concrete in section 51-6.

4. Temporary bracing installation

Replace the 1st paragraph of section 51-4.01C(2)(a) with:

04-19-19

Submit shop drawings for PC concrete members to the OSD Documents Unit unless otherwise specified.

Replace *Reserved* in section 51-4.01C(2)(e) with:

04-19-19

For PC deck panels, shop drawings must include:

1. Panel materials, shapes, and dimensions.
2. Deck panel layout identifying the locations of each panel.
3. Reinforcing, joint, and connection details.
4. Complete details of the methods, materials, and equipment used in prestressing and precasting work.
5. Type of texture and method of forming the textured finish.
6. Methods and details for lifting, bracing, and erection.
7. Method of support and grade adjustment.
8. Methods of sealing against concrete leaks.

Replace the 2nd paragraph of section 51-4.02B with:

04-19-19

Handle, store, transport, and erect PC members in a position such that the points of support and directions of the reactions with respect to the member are approximately the same as when the member is in its final position.

Replace *Reserved* in section 51-4.02D(7) with:

04-19-19

Clearly label the top surface of each panel with the word *TOP* as shown on the deck panel layout using waterproof paint or other authorized means.

Apply a coarse texture to at least 90 percent of the deck panel top surface area by brooming with a stiff bristled broom or by other suitable devices that results in uniform scoring parallel with the prestressing strands. The top surface texture must have a maximum 1/8-inch texture.

Each camber strip must:

1. Consist of high density expanded polystyrene with a minimum compressive strength of 55 psi.
2. Consist of a single layer and extend continuously under each deck panel.
3. Achieve a height that accounts for roadway profile, cross slope, and girder camber.
4. Have 1/4-inch v-notches or 1/2 by 1/2-inch slots cut into the top surface on 4-foot centers.

Camber strip dimensions must comply with the following table:

Polystyrene Camber Strip Dimensions

Height (H) (inches)	Width (W) (inches)
1 to 2.5	1.5
Greater than 2.5 and less than or equal to 3.5	1.75
Greater than 3.5 and less than or equal to 4	2

Chemical adhesive must be suitable for use with concrete and polystyrene.

Nondestructive Testing for Steel Standards and Poles

Weld location	Weld type	Minimum required NDT
Circumferential splices around the perimeter of tubular sections, poles, and arms	CJP groove weld with backing ring	100% UT or RT
Longitudinal seam	CJP or PJP groove weld	Random 25% MT
Longitudinal seam within 6 inches of a circumferential weld	CJP groove weld	100% UT or RT
Welds attaching base plates, flange plates, pole plates, or mast arm plates to poles or arm tubes	CJP groove weld with backing ring and reinforcing fillet	t ≥ 1/4 inch: 100% UT and 100% MT t < 1/4 inch: 100% MT after final weld pass
	External (top) fillet weld for socket-type connections	100% MT
Hand holes and other appurtenances	Fillet and PJP welds	MT full length on random 25% of all standards and poles
Longitudinal seam on the telescopic female end, designated slip-fit length plus 6 inches	CJP groove weld	100% UT or RT

NOTE: t = pole or arm thickness

Replace the 2nd paragraph of section 59-1.02C with:

10-19-18

Coatings selected for use must comply with the volatile organic compound concentration limits specified for the air quality district where the coating is applied. The undercoats and finish or final coats selected for use must be compatible with each other.

Add after the paragraph of section 59-2.01A(3)(a):

10-19-18

If requested by the Engineer, submit documentation from the coating manufacturer verifying the compatibility of the undercoats and finish or final coats selected for use.

AA

60 EXISTING STRUCTURES

04-19-19

Replace section 60-2.02B with:

04-19-19

60-2.02B Materials

Design criteria for temporary support shoring and temporary bracing must comply with section 48-3.02B.

Add to section 60-3.01A:

10-19-18

If you are unable to complete bridge reconstruction activities before the bridge is to be opened to traffic, furnish and maintain temporary decking under section 48-4 until that portion of the work is complete.

Replace the 3rd and 4th paragraphs of section 60-3.02C(3) with:

04-19-19

Remove asphalt concrete surfacing by cold milling under the following conditions:

1. If a membrane seal is shown:
 - 1.1. Remove the seal by cold milling
 - 1.2. Do not remove more than 1/2 inch of the existing concrete slab

2. If a membrane seal is not shown:
 - 2.1. Remove asphalt concrete surfacing until a 1/2-inch minimum of surfacing remains on top of existing concrete slab
 - 2.2. Use other authorized means to remove the remaining asphalt concrete without damage to the concrete slab

Add to section 60-3.02C(3):

04-19-19

Where a portion of the asphalt concrete surfacing is to remain, saw cut a 2-inch-deep true line along the edge to remain in place before removing asphalt concrete. Remove the asphalt concrete without damaging the surfacing to remain in place.

DIVISION VIII MISCELLANEOUS CONSTRUCTION

78 INCIDENTAL CONSTRUCTION

04-19-19

Replace section 78-4.03 with:

04-19-19

78-4.03 PAINTING CONCRETE

78-4.03A General

78-4.03A(1) Summary

Section 78-4.03 includes specifications for preparing and painting concrete surfaces.

78-4.03A(2) Definitions

Reserved

78-4.03A(3) Submittals

Submit the coating manufacturer's application instructions at least 7 days before use.

78-4.03A(4) Quality Assurance

Reserved

78-4.03B Materials

Coatings for concrete must comply with the specifications for acrylic emulsion paint for exterior masonry in section 91-4.02B.

Coatings must be white.

78-4.03C Construction

78-4.03C(1) General

Reserved

78-4.03C(2) Surface Preparation

Before painting, surfaces must be:

1. At least 28 days old.
2. Prepared under SSPC-SP 13/NACE no. 6. Pressure rinse the prepared surfaces before applying the paint.
3. Thoroughly dry. You may use artificial drying methods if authorized.

78-4.03C(3) Application

Apply at least 2 coats under the manufacturer's instructions and SSPC-PA 7. Protect adjacent surfaces during painting using an authorized method.

78-4.03D Payment

Not Used

Replace section 78-4.04 with:

04-19-19

78-4.04 STAINING CONCRETE AND SHOTCRETE

78-4.04A General

78-4.04A(1) Summary

Section 78-4.04 includes specifications for preparing and staining concrete and shotcrete surfaces.

78-4.04A(2) Definitions

acid stain: non-tintable, transparent stain that contains dilute acid.

water-based stain: semi-transparent or solid water-based coating in an acrylic emulsion vehicle, that can be tinted to match an AMS-STD-595 color.

78-4.04A(3) Submittals

78-4.04A(3)(a) General

Submit the stain and sealer manufacturer's product data and application instructions at least 7 days before starting staining activities.

78-4.04A(3)(b) Contractor Qualifications

Submit the following documentation at least 10 days before the prestaining meeting:

1. Summary of the staining contractor's experience that demonstrates compliance with section 78-4.04A(4)(c).
2. List of at least 3 projects completed in the last 5 years that demonstrate the staining contractor's ability to stain surfaces similar to the surfaces for this project. For each project include:
 - 2.1. Project description
 - 2.2. Name and phone number of the owner
 - 2.3. Staining completion date
 - 2.4. Color photos of the completed stained surface

78-4.04A(3)(c) Staining Quality Work Plan

Submit a staining quality work plan at least 10 days before the prestaining meeting. The work plan must include details for preparing and staining the surfaces to achieve the required color, and for sealing the surfaces, including:

1. Number of applications that will be used to apply the stain
2. For each application of the stain, a description of:
 - 2.1. Manufacturer, color, finish, and percentage strength mixture of the stain that will be applied
 - 2.2. Proposed methods and tools for applying the stain
3. Proposed methods for protecting adjacent surfaces during staining
4. Proposed methods and tools for applying the sealer

For acid stains, the work plan must also include a rinse water collection plan for containing all liquid, effluent, and residue resulting from preparing and staining the surfaces.

78-4.04A(4) Quality Assurance

78-4.04A(4)(a) General

Reserved

78-4.04A(4)(b) Test Panels

Stain the authorized test panel complying with section 51-1.01D(2)(c) or section 53-3.01D(3).

The test panel must be:

1. Stained using the same personnel, materials, equipment, and methods to be used in the work
2. Accessible for viewing
3. Displayed in an upright position near the work
4. Authorized for staining before starting the staining work

If ordered, construct additional test panels until a satisfactory color is attained. The preparing and staining of additional test panels is change order work.

The Engineer uses the authorized stained test panel to determine the acceptability of the stained surface.

Dispose of the test panels after the staining work is complete and authorized. Notify the Engineer before disposing of the test panels.

78-4.04A(4)(c) Contractor Qualifications

The staining contractor must have experience staining surfaces to simulate the appearance of natural rock formations or stone masonry, and must have completed at least 3 projects in the past 5 years involving staining of surfaces similar to the surfaces for this project.

78-4.04A(4)(d) Prestaining Meeting

Before starting staining activities, conduct a meeting to discuss the staining quality work plan. Meeting attendees must include the Engineer and all staining contractors.

78-4.04B Materials

78-4.04B(1) General

Reserved

78-4.04B(2) Stain

78-4.04B(2)(a) General

The stain must be:

1. Commercially available product designed specifically for exterior applications
2. Specifically manufactured for staining concrete surfaces

78-4.04B(2)(b) Acid Stain

Acid stain must:

1. Contain dilute acid that penetrates and etches the surfaces
2. Be a water-based solution of inorganic metallic salts
3. Produce abrasion-resistant color deposits

78-4.04B(2)(c) Water-based Stain

Water-based stain must be:

1. Acrylic emulsion
2. Non-fading and UV resistant
3. Capable of producing irregular, mottled tones

78-4.04B(3) Sealer

The sealer must be as recommended by the stain manufacturer, clear and colorless, and have a matte finish when dry.

78-4.04B(4) Joint Sealing Compound

Reserved

78-4.04C Construction

78-4.04C(1) General

At locations where there is exposed metal adjacent to the surfaces to be stained, seal the joint between the surfaces to be stained and the exposed metal with a joint sealing compound before applying the stain.

78-4.04C(2) Surface Preparation

Test surfaces for acceptance of the stain before applying the stain. Clean surfaces that resist accepting the stain and retest until passing.

Before staining, the surfaces must be:

1. At least 28 days old
2. Prepared under SSPC-SP 13/NACE no. 6
3. Thoroughly dry

DIVISION IX TRAFFIC CONTROL DEVICES

82 SIGNS AND MARKERS

04-19-19

Replace the list in the 1st paragraph of section 82-2.01C with:

04-19-19

1. Aluminum sheeting
2. Retroreflective sheeting
3. Color imaging methods and film
4. Protective-overlay film

Replace section 82-2.02D with:

04-19-19

82-2.02D Color Imaging Methods and Film

The material used for color imaging methods, film, and protective-overlay must be recommended by the retroreflective sheeting manufacturer.

Colored retroreflective sheeting must be used for the background.

Signs with green, red, blue, or brown backgrounds may use reverse-screened-process color on white retroreflective sheeting for the background color. The coefficient of retroreflection must be at least 70 percent of the coefficient of retroreflection specified in ASTM D4956 for the corresponding color of retroreflective sheeting.

The sign must have outdoor weatherability characteristics equivalent to those specified for the corresponding color of retroreflective sheeting in ASTM D4956.

Replace section 82-5.01A with:

10-19-18

Section 82-5 includes specifications for fabricating and installing markers, including milepost markers.

Replace the 2nd paragraph in section 82-5.02E with:

10-19-18

A target plate for milepost marker or Type L-1 (CA) or Type L-2 (CA) object marker installed on a metal post must be manufactured from an aluminum sheet or zinc-coated steel sheet.

Replace section 82-5.02H with:

10-19-18

82-5.02H Milepost Markers

Letters and numerals on a milepost marker must be made with opaque black paint or film. The paint and film must have an equivalent outdoor weatherability as the retroreflective sheeting specified in ASTM D4956. Nonreflective, opaque, black film must be vinyl or acrylic material.

Film for letters and numerals must be computer cut and have pressure-sensitive adhesive.

Replace the 5th paragraph of section 82-5.03 with:

10-19-18

Use stencils to paint letters and numerals on milepost markers.

AA

83 RAILINGS AND BARRIERS

04-19-19

Replace section 83-2.01A(3) with:

04-19-19

For midwest guardrail systems and thrie beam barrier, install steel foundation tubes and soil plates in soil.

Replace the 4th paragraph of section 83-2.03C with:

04-19-19

If median barrier delineation is shown, match the barrier marker spacing to the raised pavement marker spacing on the adjacent median edge line pavement delineation.

Replace the paragraph of section 83-3.03A(11) with:

04-19-19

Where concrete barrier markers are shown, cement the markers to the barrier under the manufacturer's instructions. Match the barrier marker spacing to the raised pavement marker spacing on the adjacent median edge line pavement delineation.

AA

84 MARKINGS

04-19-19

Replace section 84-2 with:

10-19-18

84-2 TRAFFIC STRIPES AND PAVEMENT MARKINGS

84-2.01 GENERAL

84-2.01A Summary

Section 84-2 includes specifications for applying traffic stripes and pavement markings.

Traffic stripes and pavement markings must comply with ASTM D6628 for daytime and nighttime color.

Retroreflectivity must be measured under ASTM E1710 and the sampling protocol specified in ASTM D7585.

84-2.01B Definitions

pavement marking: Transverse marking such as (1) a limit line, (2) a stop line, or (3) a word, symbol, shoulder, parking stall, or railroad-grade-crossing marking.

traffic stripe: Longitudinal centerline or lane line used for separating traffic lanes in the same direction of travel or in the opposing direction of travel or a longitudinal edge line marking the edge of the traveled way or the edge of a lane at a gore area separating traffic at an exit or entrance ramp. A traffic stripe is shown as a traffic line.

84-2.01C Submittals

For each lot or batch of traffic stripe material, primer, and glass beads, submit:

1. Certificate of compliance, including the material name, lot or batch number, and manufacture date
2. METS notification letter stating that the material is authorized for use, except for thermoplastic and primer
3. SDS
4. Manufacturer's Instructions

For each lot or batch of thermoplastic, submit a manufacturer's certificate of compliance and the following test results from the California Test 423:

1. Brookfield Thermosel viscosity
2. Hardness
3. Yellowness index, white only
4. Daytime luminance factor
5. Yellow color, yellow only
6. Glass bead content
7. Binder content

The date of the test must be within 1 year of use.

Submit test results for each lot of beads specifying the EPA test methods used and tracing the lot to the specific test sample. The testing for lead and arsenic content must be performed by an independent testing laboratory.

Submit the thermoplastic test stripe to the Engineer.

Submit the retroreflectivity test result within 5 days of testing the traffic stripes and pavement markings. The data must include the retroreflectivity, time, date, and GPS coordinates for each measurement.

84-2.01D Quality Assurance

84-2.01D(1) General

Reserved

84-2.01D(2) Quality Control

Before starting permanent application of methyl methacrylate and two component paint traffic stripes and pavement markings, apply a test stripe on roofing felt or other suitable material in the presence of the Engineer. The test stripe section must be at least 50 feet in length.

Upon request, apply a thermoplastic test stripe on suitable material in the presence of the Engineer during the application of thermoplastic traffic stripes or markings. The test stripe must be at least 1 foot in length.

Remove loose glass beads before measuring the retroreflectivity. Obtain authorization to proceed with the application of traffic stripes and pavement markings.

Within 30 days of application, test the traffic stripes and pavement markings under the test methods and frequencies shown in the following table:

Traffic Stripe Testing Frequency

Quality characteristic	Test method	Minimum sampling and testing frequency
Initial retroreflectivity (min, $\text{mcd}\cdot\text{m}^{-2}\cdot\text{lx}^{-1}$)	ASTM E1710	ASTM D7585 ^a
White		
Yellow		

^aUse the referee evaluation protocol for project length less than 10 miles. For project lengths greater than or equal to 10 miles, add one evaluation for every additional mile.

Verify the glass bead application rate by stabbing the glass bead tank with a calibrated rod.

84-2.01D(3) Department Acceptance

The Engineer will perform a nighttime, drive-through, visual inspection of the retroreflectivity of the traffic stripes and pavement markings and notify you of any locations with deficient retroreflectivity. Test the retroreflectivity of the deficient areas to confirm striping and pavement markings meets the requirements.

The thermoplastic test stripe will be tested for yellow color, daytime luminance factor, and yellowness index requirements by METS.

84-2.02 MATERIALS

84-2.02A General

Reserved

84-2.02B Glass Beads

Each lot of glass beads must comply with EPA Test Method 3052 and 6010B or 6010C. Glass beads must contain less than 200 ppm each of arsenic and lead.

Type 1 glass beads must comply with AASHTO M 247.

Type 2 glass beads must comply with AASHTO M 247. At least 75 percent of the beads by count must be true spheres that are colorless and do not exhibit dark spots, air inclusions, or surface scratches when viewed under 20X magnification.

High-performance glass beads must be on the Authorized Material List for high-performance glass beads.

Large-gradation glass beads must be on the Authorized Material List for two component traffic paint.

Glass beads for methyl methacrylate must be on the Authorized Material List for methyl methacrylate traffic striping and pavement marking.

Glass beads for paint must comply with State Specification 8010-004.

Glass beads must be surface treated, according to the bead and the material manufacturer's instructions, to promote adhesion with the specified material.

84-2.02C Thermoplastic

Thermoplastic must comply with State Specification PTH-02HYDRO, or PTH-02ALKYD.

Sprayable thermoplastic must comply with State Specification PTH-02SPRAY.

Each lot or batch of thermoplastic must be tested under California Test 423.

84-2.02D Methyl Methacrylate

Methyl methacrylate traffic paint must:

1. Be on the Authorized Material List for methyl methacrylate traffic striping and pavement marking
2. Be Category 2

84-2.02E Traffic Striping and Pavement Marking Tape

Traffic striping and pavement marking tape must be on the Authorized Material List for signing and delineation materials.

04-19-19

White tape must have an initial retroreflectivity of a minimum 700 mcd/m².

Yellow tape must have an initial retroreflectivity of a minimum 500 mcd/m².

10-19-18

When contrast is required for traffic striping and pavement marking tape, the tape must be pre-formed and retroreflective, consisting of a white film with retroreflective beads and a contrasting black film border. The contrasting black border must be a nonreflective film bonded on each side of the white film to form a continuous roll. Each black border must be a minimum of 2 inches wide. The width of the tape must be at least 4 inches wider than the stripe width.

84-2.02F Two-Component Paint

Two-component traffic paint must be on the Authorized Material List for two component traffic paint.

84-2.02G Paint

Paint must comply with the requirements shown in following table:

Paint Specifications

Paint type	Color	Specification
Waterborne traffic line	White, yellow, and black	State Specification PTWB-01R2
Waterborne traffic line for the international symbol of accessibility and other curb markings	Blue, red, and green	Federal Specification TT-P-1952E

84-2.02H–84-2.02L Reserved

84-2.03 CONSTRUCTION

84-2.03A General

Establish the alignment for traffic stripes and the layouts for pavement markings with a device or method that will not conflict with other traffic control devices.

Protect existing retroreflective pavement markers during work activities.

Remove existing pavement markers that are coated or damaged by work activities and replace with an equivalent marker on the Authorized Material List for signing and delineation materials.

A completed traffic stripe or pavement marking must:

1. Have well defined edges
2. Be uniform
3. Be free from runs, bubbles, craters, drag marks, stretch marks, and debris

A completed traffic stripe must:

1. Be straight on a tangent alignment
2. Be a true arc on a curved alignment
3. Not deviate from the width shown by more than:
 - 3.1. 1/4 inch on a tangent alignment
 - 3.2. 1/2 inch on a curved alignment

The length of the gaps and individual stripes that form a broken traffic stripe must not deviate by more than 2 inches from the lengths shown. The gaps and stripes must be uniform throughout the entire length of the traffic stripe.

Protect newly placed traffic stripes and pavement markings from traffic and work activities until the traffic stripes and pavement markings are dry or hard enough to bear traffic.

Use mechanical methods to remove dirt, contaminants, and loose material from the pavement surface before applying the traffic stripe or pavement marking.

Use abrasive blast cleaning to remove laitance and curing compound from the surface of new concrete pavement before applying the traffic stripe or pavement marking.

Construct recesses as shown in the following table:

Recess Depth Requirements

Material	Requirement	
	Depth (mils)	Depth (in)
Thermoplastic	375	3/8
Two component traffic paint	250	1/4
Methyl methacrylate traffic paint	250	1/4

Construct recesses for double traffic stripes in a single pass.

Before applying the traffic stripes and pavement markings:

1. Allow wet ground recesses to dry a minimum of 24 hours

2. Remove all powdery residue from dry recess
3. Keep the recesses dry and free from debris

Apply traffic stripes and pavement markings before the end of the same work shift.

84-2.03B Application of Traffic Stripes and Pavement Markings

84-2.03B(1) General

Apply material for a pavement marking with a stencil or a preformed marking.

Immediately remove drips, overspray, improper markings, or material tracked by traffic, using an authorized method.

Apply a traffic stripe or a pavement marking only to a clean, dry surface during a period when the pavement surface temperature is above 50 degrees F.

Apply traffic stripe or pavement marking and glass beads in a single pass. You may apply the glass beads by hand on pavement markings.

Embed glass beads to a depth of 1/2 their diameters.

Distribute glass beads uniformly on traffic stripe and pavement markings.

Glass beads with integral color must match the color of the stripe or pavement marking.

Apply glass beads with two separate applicator guns when two gradations are specified.

Allow enough overlap distance between new and existing striping patterns to ensure continuity at the start and end of the transition.

The retroreflectivity of applied traffic stripes and pavement markings must comply with the requirements shown in the following table:

Retroreflectivity Requirements		
Traffic stripe material	White (min, mcd·m ⁻² ·lx ⁻¹)	Yellow (min, mcd·m ⁻² ·lx ⁻¹)
Paint	250	125
Thermoplastic	250	125
Thermoplastic with wet night enhanced visibility	700	500
Two component	250	125
Methyl methacrylate	500	300
Tape	700	500

84-2.03B(2) Thermoplastic

84-2.03B(2)(a) General

Apply primer or surface preparation adhesive under the manufacturer's instructions:

1. To all roadway surfaces except for asphaltic surfaces less than 6 months old
2. At a minimum rate of 1 gallon per 300 square feet
3. To allow time for the thermoplastic primer to dry and become tacky before application of the thermoplastic

Do not thin the primer.

Preheat thermoplastic using preheaters with mixers having a 360-degree rotation.

Apply thermoplastic in a single uniform layer by spray or extrusion methods.

Completely coat and fill voids in the pavement surface with the thermoplastic.

Apply recessed thermoplastic at a thickness so that the top is 0 to 1/16 inch below the pavement surface.

84-2.03B(2)(b) Extruded Thermoplastic

Apply extruded thermoplastic at a temperature of 400 to 425 degrees F or as recommended by the manufacturer.

Apply extruded thermoplastic for a traffic stripe at a rate of at least 0.36 lb of thermoplastic per foot of 6-inch-wide solid stripe. The applied traffic stripe must be at least 0.060 inch thick.

Apply extruded thermoplastic pavement markings at a thickness from 0.100 to 0.150 inch.

Apply Type 2 glass beads to the surface of the molten thermoplastic at a rate of at least 8 lb of beads per 100 sq ft.

84-2.03B(2)(c) Sprayable Thermoplastic

Apply sprayable thermoplastic at a temperature of 350 to 400 degrees F.

Apply sprayable thermoplastic for a traffic stripe at a rate of at least 0.24 lb of thermoplastic per foot of 6-inch-wide solid stripe. The applied stripe must be at least 0.040 inch thick.

84-2.03B(2)(d) Thermoplastic with Enhanced Wet-Night Visibility

Apply a thermoplastic traffic stripe or pavement marking with enhanced wet-night visibility in a single pass and in the following order:

1. Uniform layer of extruded thermoplastic
2. Layer of high-performance glass beads
3. Layer of Type 2 glass beads

Apply thermoplastic with enhanced wet-night visibility at a maximum speed of 8 mph.

Apply thermoplastic with enhanced wet-night visibility for a traffic stripe at a rate of at least 0.47 lb of thermoplastic per foot of 6-inch-wide solid stripe. The applied stripe must be at least 0.090 inch thick.

Apply thermoplastic with enhanced wet-night visibility for a pavement marking at a rate of at least 1.06 lb of thermoplastic per square foot of marking. The applied pavement marking must be at least 0.100 inch thick.

Apply high-performance glass beads at a rate of at least 6 lb of glass beads per 100 sq ft of stripe or marking. Apply Type 2, glass beads at a rate of at least 8 lb of glass beads per 100 sq ft of stripe or marking.

84-2.03B(3) Methyl Methacrylate

Apply the methyl methacrylate when the pavement surface and atmospheric temperatures are from 40 to 104 degrees F.

Apply methyl methacrylate paint at a minimum thickness of 0.090 inch.

Apply recessed methyl methacrylate paint at a minimum thickness of 0.200 inch.

Apply the glass beads recommended by the methyl methacrylate manufacturer.

84-2.03B(4) Traffic Striping and Pavement Marking Tape

Do not use traffic stripe and pavement marking tape on existing open graded friction course or chip seal.

Prepare pavement surface and use primer under the traffic tape manufacturer's written instructions. Apply tape to clean and dry pavement surface. Roll or tamp the traffic tape in place.

84-2.03B(5) Two-Component Paint

Apply a two-component painted traffic stripe or pavement marking in a single pass and in the following order:

1. Coat of two-component paint
2. Application of large gradation glass beads recommended by the two-component paint manufacturer
3. Application of Type 1 glass beads

Apply two-component paint when the pavement surface temperature is above 39 degrees F and the atmospheric temperature is above 36 degrees F. The temperature of the paint must comply with the paint manufacturer's instructions.

Apply two-component paint and glass beads at a maximum speed of 10 mph.

Apply large-gradation glass beads at a minimum rate of 11.7 lb of beads per gallon of paint.

Apply Type 1 glass beads at a minimum rate of 8.3 lb of beads per gallon of paint.

Apply two-component paint for the traffic stripes and pavement markings at the thickness and application rates shown in the following table:

Type of pavement	Stripe thickness (min, inch)	Application rate (min, sq ft/gal)
HMA open graded/chip seal	0.025	64
HMA dense graded	0.020	80
Concrete	0.020	80

Apply recessed two-component paint at a thickness between 0.020 and 0.025 inch.

84-2.03B(6) Paint

Do not apply paint if:

1. Fresh paint could become damaged by rain, fog, or condensation
2. Atmospheric temperature could drop below 50 degrees F during the drying period

Do not thin paint.

Use mechanical means to paint traffic stripes and pavement markings and to apply glass beads for traffic stripes.

The striping machine must be capable of superimposing successive coats of paint on the 1st coat and on existing stripes at a minimum speed of 5 mph.

Where the configuration or location of a traffic stripe is such that the use of a striping machine is not practicable, you may apply the traffic paint and glass beads by other methods and equipment if authorized.

Apply traffic stripes and pavement markings in 1 coat on existing pavement surfaces, at an approximate rate of 107 sq ft/gal.

Apply traffic stripes and pavement markings in 2 coats on a new pavement surface. The 1st coat of paint must be dry before applying the 2nd coat.

Apply 2-coat paint at the approximate rate of 215 sq ft/gal for each coat.

Paint a 1-coat, 3-inch-wide black stripe between the two 6-inch-wide yellow stripes of a double traffic stripe. If the two 6-inch-wide yellow stripes are applied in 2 coats, apply the black stripe concurrently with the 2nd coat of the yellow stripes.

On 2-lane highways:

1. If the 1st coat of the centerline stripe is applied in the same direction as increasing post miles, use the right-hand spray gun of the 3 spray guns to apply a single yellow stripe
2. If the 1st coat of the centerline stripe is applied in the same direction as decreasing post miles, use the left-hand spray gun of the 3 spray guns to apply a single yellow stripe
3. Apply the 2nd coat of centerline striping in the opposite direction of the 1st coat

Apply glass beads at an approximate rate of 5 lb of beads per gallon of paint.

Verify the application rate of paint by stabbing the paint tank with a calibrated rod. If the striping machine has paint gauges, the Engineer may measure the volume of paint using the gauges instead of stabbing the paint tank with a calibrated rod.

84-2.03B(7) Contrast Striping

04-19-19

Contrast striping consists of black striping placed on each side of a white stripe.

10-19-18

You may use permanent tape instead of paint or thermoplastic.

Apply contrast stripe paint in one coat.

Do not use glass beads or other reflective elements in contrast striping material.

04-19-19

84-2.03B(8)–84-2.03B(10) Reserved

10-19-18

84-2.04 PAYMENT

The payment quantity for a traffic stripe is the length measured along the line of the traffic stripe without deductions for gaps in the broken traffic stripe.

The payment quantity for a pavement marking is the area covered.

A double traffic stripe consisting of two 6-inch-wide yellow stripes are measured as 2 traffic stripes except for painted traffic stripes and sprayable thermoplastic traffic stripes. A double sprayable thermoplastic traffic stripe consisting of two 6-inch-wide yellow stripes are measured as single traffic stripe.

A double painted traffic stripe consisting of two 6-inch-wide yellow stripes separated by a 3-inch-wide black stripe is measured as a single traffic stripe.

The payment quantity for contrast striping is the length measured along the line of the traffic stripe without deductions for gaps in the broken traffic stripe.

Replace section 84-9 with:

10-19-18

84-9 EXISTING MARKINGS

84-9.01 GENERAL

84-9.01A Summary

Section 84-9 includes specifications for removing existing markings.

Work performed on existing markings must comply with section 15.

84-9.01B Definitions

Reserved

04-19-19

84-9.01C Submittals

10-19-18

Submit your proposed method for removing traffic stripes and pavement markings at least 7 days before starting the removal work. Allow 2 business days for the review.

84-9.02 MATERIALS

Not Used

84-9.03 CONSTRUCTION

84-9.03A General

Remove existing traffic stripes before making any changes to the traffic pattern.

CALiPER: Commercially Available LED Product Evaluation and Reporting. A U.S. Department of Energy program that individually tests and provides unbiased information on the performance of commercially available LED luminaires and lights.

controller assembly: Assembly for controlling a system's operations, consisting of a controller unit and auxiliary equipment housed in a waterproof cabinet.

controller unit: Part of the controller assembly performing the basic timing and logic functions.

correlated color temperature: Absolute temperature in kelvin of a blackbody whose chromaticity most nearly resembles that of the light source.

detector: Detector as defined in the *California MUTCD*.

electrolier: Assembly of a lighting standard and luminaire.

flasher: Device for opening and closing signal circuits at a repetitive rate.

illuminance gradient: Ratio of the minimum illuminance on a 1-foot square of sign panel to that on an adjacent 1-foot square of sign panel.

inductive loop detector: Detector capable of being actuated by an inductance change caused by a vehicle passing or standing over the loop. An inductive loop detector includes a loop or group of loops installed in the roadway and a lead-in cable installed and connected inside a controller cabinet.

junction temperature: Temperature of the electronic junction of the LED device. The junction temperature is critical in determining photometric performance, estimating operational life, and preventing catastrophic failure of the LED.

L70: Extrapolated life in hours of the luminaire when the luminous output depreciates 30 percent from the initial values.

lighting standard: Pole and mast arm supporting the luminaire.

link: Part of a system which provides a data connection between a transmitter and receiver.

LM-79: Test method from the Illumination Engineering Society of North America specifying the test conditions, measurements, and report format for testing solid state lighting devices, including LED luminaires.

LM-80: Test method from the Illumination Engineering Society of North America specifying the test conditions, measurements, and report format for testing and estimating the long-term performance of LEDs for general lighting purposes.

luminaire: Assembly that houses the light source and controls the light emitted from the light source.

mid-span access method: Procedure in which fibers from a single buffer tube are accessed and spliced to a multi buffer tube cable without cutting the unused fibers in the buffer tube, or disturbing the remaining buffer tubes in the cable.

National Voluntary Laboratory Accreditation Program: U.S. Department of Energy program that accredits independent testing laboratories.

optical time domain reflectometer: Fiber optic test equipment that is used to measure the total amount of power loss between two points and over the corresponding distance. It provides a visual and printed display of the relative location of system components such as fiber sections, splices and connectors as well as the losses that are attributed to each component and or defects in the fiber.

pedestrian change interval: Pedestrian change interval as defined in the *California MUTCD*.

powder coating: Coating applied electrostatically using exterior-grade, UV-stable, polymer powder.

power factor: Ratio of the real power component to the complex power component.

power meter: Portable fiber optic test equipment that, when coupled with a light source, is used to perform end-to-end attenuation testing. Its display indicates the amount of power injected by the light

source at the designed wavelength of the system under testing that arrives at the receiving end of the link.

pretimed controller assembly: Assembly operating traffic signals under a predetermined cycle length.

programming mechanism: Device to program the accessible pedestrian signal operation.

pull box: Box with a cover that is installed in an accessible place in a conduit run to facilitate the pulling in of wires or cables.

push button information message: Push button information message as defined in the *California MUTCD*.

push button locator tone: Push button locator tone as defined in the *California MUTCD*.

segment: Continuous cable terminated by 2 splices, 2 connectors or 1 splice and 1 connector.

signal face: Signal face as defined in the *California MUTCD*.

signal head: Signal head as defined in the *California MUTCD*.

signal indication: Signal indication as defined in the *California MUTCD*.

signal section: Signal section as defined in the *California MUTCD*.

signal standard: Pole with or without mast arms carrying 1 or more signal faces.

street side lumens: Lumens from a luminaire directed to light up areas between the fixture and the roadway, such as traveled ways and freeway lanes.

surge protection device: Subsystem or component that protects equipment against short-duration voltage transients in power line.

total harmonic distortion: Ratio of the rms value of the sum of the squared individual harmonic amplitudes to the rms value of the fundamental frequency of a complex waveform.

traffic-actuated controller assembly: Assembly for operating traffic signals under the varying demands of traffic as registered by detector actuation.

traffic phase: Traffic phase as defined in the *California MUTCD*.

vehicle: Vehicle as defined in the *California Vehicle Code*.

vibrotactile pedestrian device: Vibrotactile pedestrian device as defined in the *California MUTCD*.

10-19-18

Delete the 9th and 10th paragraphs of section 86-1.01C(1).

Replace section 86-1.01C(3) with:

10-19-18

86-1.01C(3) Luminaires

Submit for a luminaire:

1. Maximum power in watts
2. Maximum designed junction temperature
3. Heat sink area in square inches
4. Designed junction-to-ambient thermal resistance calculation with thermal resistance components clearly defined
5. L70 in hours when extrapolated for the average nighttime operating temperature
6. Life expectancy based on the junction temperature
7. Manufacturer's data sheet for the power supply, including the rated life

Submit the manufacturer's QC test data for luminaires as an informational submittal.

Replace section 86-1.01C(4) with:

10-19-18

86-1.01C(4) Reserved

Replace the 3rd paragraph of section 86-1.02B(1) with:

04-19-19

Conduit used for horizontal directional drilling must be high density polyethylene Type IPS, SDR 9 and comply with ASTM F2160.

Replace the 8th paragraph of section 86-1.02B(1) with:

10-19-18

High density polyethylene for innerduct must:

1. Comply with ASTM D3485, D3035, D2239, and D2447, and NEMA TC7 and TC2
2. Have a minimum tensile yield strength of 3300 psi under ASTM D638
3. Have a density of $59.6187 \text{ lb/ft}^3 \pm 0.3121 \text{ lb/ft}^3$ under ASTM D1505

04-19-19

Replace the 9th paragraph of section 86-1.02B(1) with:

04-19-19

Tracer wire must be a minimum no. 12 solid copper conductor with orange insulation Type TW, THW, RHW, or USE. For direct burial, the tracer wire insulation must be Type UF.

Replace the 4th paragraph of section 86-1.02C(1) with:

10-19-18

The cover marking must include CALTRANS and one of the following:

1. *SERVICE* for service circuits between a service point and service disconnect
2. *SERVICE IRRIGATION* for circuits from a service equipment enclosure to an irrigation controller
3. *SERVICE BOOSTER PUMP* for circuits from a service equipment enclosure to the booster pump
4. *TDC POWER* for circuits from a service equipment enclosure to telephone demarcation cabinet
5. *LIGHTING* for a lighting system
6. *SIGN ILLUMINATION* for a sign illumination system
7. *SIGNAL AND LIGHTING* for a signal and lighting system
8. *RAMP METER* for a ramp metering system
9. *TMS* for a traffic monitoring station
10. *FLASHING BEACON* for a flashing beacon system
11. *CMS* for a changeable message sign system
12. *INTERCONNECT* for an interconnect conduit and cable system
13. *FIBER OPTIC* for fiber optic cable system
14. *ELECTRICAL SYSTEMS* if more than one system is shared in the same pull box

10-19-18

Delete the 3rd paragraph of section 86-1.02C(2).

Replace the 1st and 2nd paragraphs of section 86-1.02C(3) with:

10-19-18

A traffic pull box and cover must comply with AASHTO HS20-44 and load tested under AASHTO M 306.

The frame must be anchored to the box with 2-1/4-inch-long concrete anchors with a 1/4 inch diameter. A no. 3-1/2(T) pull box must have 4 concrete anchors, one placed in each corner. No. 5(T) and no. 6(T) pull boxes must have 6 concrete anchors, one placed in each corner and one near the middle of each of the longer sides.

Replace section 86-1.02C(4)(b) with:

10-19-18

86-1.02C(4)(b) Tamper-Resistant Nontraffic Pull Box

86-1.02C(4)(b)(i) General

A tamper resistant nontraffic pull box must include a pull box with one of the following:

1. Anchored cover
2. Lockable cover
3. Pull box insert

86-1.02C(4)(b)(ii) Anchored Cover

The anchored cover must:

1. Be of 1/2-inch-thick mild steel, hot dip galvanized, post fabrication.
2. Be hot dip galvanized after manufacturing with spikes removed from the galvanized surfaces.
3. Have a center space for a top lock nut that must be torqued to 200 ft-lb.
4. Have a center opening for a stainless steel threaded cap to cover the lock nut.
5. Weigh a minimum of 85 lb.
6. Include an all-around security skirt of 1/4-inch thick steel. The skirt must be sized to encase a nontraffic pull box or sized to fit within a traffic pull box.
7. Be welded to the skirt.

86-1.02C(4)(b)(iii) Lockable Cover

The lockable cover must:

1. Be manufactured from minimum 3/16-inch-thick galvanized steel or a polymer of minimum strength equal to 3/16 inch steel.
2. Be secured to the pull box with a locking mechanism of equal or greater strength than the manufactured material.
3. Have 1/2-by-2-inch slot holes for lifting.
4. Have dimensions complying with one of the following:
 - 4.1. Department's standards for pull box covers as shown if the lockable cover is secured to the inside lip of the pull box.
 - 4.2. Department's standards for the length and width as shown for pull box covers if the lockable cover is secured to the top of the pull box.

86-1.02C(4)(b)(iv) Pull Box Insert

The pull box insert must:

1. Be made of minimum 3/16-inch-thick or 10 gauge mild hot-dipped galvanized steel
2. Have a minimum of 2 mounting brackets that rest under the side or end wall
3. Be lockable with a padlock having a minimum 3/8-inch shackle
4. Have dimensions complying with the Department's standards for the length and width as shown for pull box covers

Delete section 86-1.02C(4)(d).

10-19-18

Delete section 86-1.02C(4)(e).

10-19-18

Delete section 86-1.02C(4)(f).

10-19-18

Replace section 86-1.02D(3) with:

10-19-18

86-1.02D(3) Warning Tape

Warning tape must be orange color polyolefin film, minimum elongation of 500 percent before breakage, water and corrosion resistant, and comply with requirements shown in the following table:

Warning Tape Requirements

Quality characteristic	Requirement
Thickness (min, mil)	4
Width (in)	4
Tensile strength of material (min, psi)	2800
Message spacing intervals (ft)	3

The warning tape must have a printed message that reads: *CAUTION: CALTRANS FACILITIES BELOW.*

The printed text height and color must be 1 inch, black color text over bright orange background.

Replace the 2nd paragraph of section 86-1.02E with:

10-19-18

Each sensor must:

1. Have a dissipation factor less than 0.04 nF when measured in the 20 nF range
2. Have resistance greater than 20 Megaohms
3. Be 1/4 inch wide by 6 feet long by 1/16 inch thick
4. Have a RG-58C/U coaxial screen transmission cable, jacketed with high-density polyethylene, rated for direct burial and resistant to nicks and cuts
5. Operate over a temperature range from -40 to 160 degrees F
6. Have a signal to noise ratio equal to or greater than 10 to 1
7. Have an output signal of a minimum 250 mV \pm 20 percent for a wheel load of 400 lb at 55 mph and 70 degrees F
8. Have an insulation resistance greater than 500 M Ω
9. Have a life cycle of a minimum 25 million equivalent single axle loadings

Replace section 86-1.02F(1) with:

10-19-18

86-1.02F(1) General

Conductors and cables must be clearly and permanently marked the entire length of their outer surface with:

1. Manufacturer's name or trademark
2. Insulation-type letter designation

3. Conductor size
4. Voltage
5. Number of conductors for a cable

The minimum insulation thickness and color code requirements must comply with NEC.

Replace the 2nd paragraph of section 86-1.02F(2)(a) with:

10-19-18

Conductors must be identified as shown in the following table:

Conductor Identification

Circuit	Signal phase or function	Identification		Band symbols	Copper size
		Insulation color			
		Base	Stripe ^a		

Signals (vehicle) ^{a,b}	2, 6	Red, yellow, brown	Black	2, 6	14
	4, 8	Red, yellow, brown	Orange	4, 8	14
	1, 5	Red, yellow, brown	None	1, 5	14
	3, 7	Red, yellow, brown	Purple	3, 7	14
	Ramp meter 1	Red, yellow, brown	None	No band required	14
	Ramp meter 2	Red, yellow, brown	Black	No band required	14
Pedestrian signals	2p, 6p	Red, brown	Black	2p, 6p	14
	4p, 8p	Red, brown	Orange	4p, 8p	14
	1p, 5p	Red, brown	None	1p, 5p	14
	3p, 7p	Red, brown	Purple	3p, 7p	14
Push button assembly or accessible pedestrian signal	2p, 6p	Blue	Black	P-2, P-6	14
	4p, 8p	Blue	Orange	P-4, P-8	14
	1p, 5p	Blue	None	P-1, P-5	14
	3p, 7p	Blue	Purple	P-3, P-7	14
Traffic signal controller cabinet	Ungrounded circuit conductor	Black	None	CON-1	6
	Grounded circuit conductor	White	None	CON-2	6
Highway lighting pull box to luminaire	Ungrounded - line 1	Black	None	No band required	14
	Ungrounded - line 2	Red	None	No band required	14
	Grounded	White	None	No band required	14
Multiple highway lighting	Ungrounded - line 1	Black	None	ML1	10
	Ungrounded - line 2	Red	None	ML2	10
	Ungrounded - line 3	White	None	ML3	10
Lighting control	Ungrounded - Photoelectric unit	Black	None	C1	14
	Switching leg from Photoelectric unit or SM transformer	Red	None	C2	14
Service	Ungrounded - line 1 (signals)	Black	None	No band required	6
	Ungrounded - line 2 (lighting)	Red	None	No band required	8
Sign lighting	Ungrounded - line 1	Black	None	SL-1	10
	Ungrounded - line 2	Red	None	SL-2	10
Flashing beacons	Ungrounded between flasher and beacons	Red or yellow	None	FB-Location. ^c	14
Grounded circuit conductor	Push button assembly or accessible pedestrian signal	White	Black	No band required	14
	Signals and multiple lighting	White	None	No band required	10
	Flashing beacons and sign lighting	White	None	No band required	12
	Lighting control	White	None	C-3	14
	Service	White	None	No band required	14

Railroad preemption		Black	None	R	14
Spares		Black	None	No band required	14

Notes:

^aOn overlaps, the insulation is striped for the 1st phase in the designation, e.g., phase (2+3) conductor is striped as for phase 2.

^bBand for overlap and special phases as required

^cFlashing beacons having separate service do not require banding.

10-19-18

Delete the 4th paragraph of section 86-1.02F(2)(a).

Replace the 2nd paragraph of section 86-1.02F(2)(c)(ii) with:

10-19-18

An equipment grounding conductor must be insulated.

Replace the 3rd paragraph of section 86-1.02F(3)(d)(ii) with:

10-19-18

Cable must comply with the requirements shown in the following table:

Cable type	Conductor quantity and type	Cable jacket thickness (mils)		Maximum nominal outside diameter (inch)	Conductor color code
		Average	Minimum		

3CSC	3 no. 14	44	36	0.40	Blue/black stripe, blue/orange stripe, white/black stripe
5CSC	5 no. 14	44	36	0.50	Red, yellow, brown, black, white
9CSC	1 no. 12 8 no. 14	60	48	0.65	No. 12 - white, No. 14 - red, yellow, brown, black, red/black stripe, yellow/black stripe, brown/black stripe, white/black stripe
12CSC	1 no. 12 11 no. 14	60	48	0.80	No. 12 - white No. 14 - red, yellow, brown, black, red/black stripe, yellow/black stripe, brown/black stripe, black/red stripe, black/white stripe, red/white stripe, brown/white stripe
28CSC	1 no. 10 27 no. 14	80	64	0.90	No. 10 - white No. 14 - red/black stripe, yellow/black stripe, brown/black stripe, red/orange stripe, yellow/orange stripe, brown/orange stripe, red/silver stripe, yellow/silver stripe, brown/silver stripe, red/purple stripe, yellow/purple stripe, brown/purple stripe, red/2 black stripes, brown/2 black stripes, red/2 orange stripes, brown/2 orange stripes, red/2 silver stripes, brown/2 silver stripes, red/2 purple stripes, brown/2 purple stripes, blue/black stripe, blue/orange stripe, blue/silver stripe, blue/purple stripe, white/black stripe, black/red stripe, black

Replace the 3rd paragraph of section 86-1.02G with:

10-19-18

The self-adhesive reflective labels must:

1. Be from 3 to 5 mils thick
2. Have all black capital characters on a white background
3. Extend beyond the character by a minimum of 1/4 inch

Replace the 4th paragraph of section 86-1.02H with:

10-19-18

PVC electrical tape must have a minimum thickness of 6 mils.

Replace section 86-1.02K with:

10-19-18

86-1.02K Luminaires

86-1.02K(1) General

A luminaire must:

1. Be self-contained, not requiring assembly.
2. Comply with UL 1598 for luminaires in wet locations.
3. Have a power supply with ANSI/IEC rating of at least IP65.
4. Weigh less than 35 lb.
5. Have a minimum operating life of 100,000 hours when operated for an average time of 11.5 hours at an average temperature of 70 degrees F.
6. Operate over a temperature range from -40 to 130 degrees F.
7. Be operationally compatible with photoelectric controls.
8. Have a correlated color temperature range from 2700 to 3500 K and a color rendering index of 70 or greater.
9. Have a maximum-effective projected area of 1.4 sq ft when viewed from either side or end.
10. Comply with California Test 611.
11. Have a power factor of 0.90 or greater. The total harmonic distortion, current, and voltage induced into a power line by a luminaire must not exceed 20 percent.
12. Comply with the maximum power consumption and isofootcandle curves as shown.
13. Be on the Authorized Material List for LED luminaires or must be submitted for testing and addition to the AML.

A luminaire must include a surge protection device to withstand high-repetition noise transients caused by utility line switching, nearby lightning strikes, and other interferences. The device must protect the luminaire from damage and failure due to transient voltages and currents as defined in Tables 1 and 4 of ANSI/IEEE C64.41.2 for location category C-High. The surge protection device must comply with UL 1449 and ANSI/IEEE C62.45 based on ANSI/IEEE C62.41.2 definitions for standard and optional waveforms for location category C-High.

The luminaire must operate over the entire voltage range from 120 to 480 V(ac), 60 ± 3 Hz or one of the following:

1. From 95 to 277 V(ac) for luminaires rated 120 V(ac) or 240 V(ac)
2. From 347 to 480 V(ac) for luminaires rated 480 V(ac)

The fluctuations of line voltage must have no visible effect on the luminous output.

The L70 of the luminaire must be the minimum operating life or greater. Illuminance measurements must be calibrated to standard photopic calibrations.

The luminaire's housing must withstand a 1008 hour cyclic salt fog spray/UV test under ASTM D5894 and an evaluation under ASTM D714 with a blister size of 8 or greater and no more than medium density.

The luminaire's housing must be marine-grade alloy with less than 0.2 percent copper or die cast aluminum. All exposed aluminum must be anodized. A chromate conversion undercoating must be used underneath a thermoplastic polyester powder coat.

External bolts, screws, hinges, hinge pins, and door closure devices must be corrosion resistant.

The housing must be designed to prevent the buildup of water on its top surface. Exposed heat sink fins must be oriented to allow water to run off the luminaire and carry dust and other accumulated debris away from the unit. The optical assembly of the luminaire must be protected against dust and moisture intrusion to at least an UL 60529 rating of IP66. The power supply enclosure must be protected to at least an UL 60529 rating of IP43.

If the components are mounted on a down-opening door, the door must be hinged and secured to the luminaire's housing separately from other components. The door must be secured to the housing to prevent accidental opening. A safety cable must mechanically connect the door to the housing.

A luminaire must have a barrier-type terminal block secured to the housing to connect field wires. The terminal screws must be captive and equipped with wire grips for conductors up to no. 6.

The conductors and terminals must be identified and marked.

If needed, each refractor or lens must be made of UV-inhibiting high-impact plastic, such as acrylic or polycarbonate, or heat and impact-resistant glass. The refractor or lens must be resistant to scratching. Polymeric materials, except for the lenses of enclosures containing either the power supply or electronic components of the luminaire, must be made of UL94 V-0 flame-retardant materials.

The luminaire must be permanently marked inside the unit and outside of its packaging box. Marking consists of:

1. Manufacturer's name or trademark
2. Month and year of manufacture
3. Model, serial, and lot numbers
4. Rated voltage, wattage, and power in VA

An LED luminaire must:

1. Comply with Class A emission limits under 47 CFR 15(B) for the emission of electronic noise.
2. Have a power supply with:
 - 2.1. 2 leads to accept standard 0-10 V(dc).
 - 2.2. Dimming control compatible with IEC 60929, Annex E. If the control leads are open or the analog control signal is lost, the circuit must default to 100-percent power.
 - 2.3. Case temperature self rise of 77 degrees F or less above ambient temperature in free air with no additional heat sinks.
3. Have passive thermal management with enough capacity to ensure proper heat dissipation and functioning of the luminaire over its minimum operating life. The maximum junction temperature for the minimum operating life must not exceed 221 degrees F.
4. Have a junction-to-ambient thermal resistance of 95 degrees F per watt or less.
5. Contain circuitry that automatically reduces the power to the LEDs so the maximum junction temperature is not exceeded when the ambient temperature is 100 degrees F or greater.
6. Have a heat sink made of aluminum or other material of equal or lower thermal resistance. The use of fans or other mechanical devices is not allowed for cooling the luminaire.

The catastrophic loss or failure of 1 LED must not result in the loss of more than 20 percent of the total luminous output of the LED luminaire.

86-1.02K(2) Roadway Luminaires

A roadway luminaire must:

1. Have a housing color that matches a color no. 26152 to 26440, 36231 to 36375, or 36440 of AMS-STD-595
2. Have an ANSI C136.41-compliant, locking-type, photocontrol receptacle with dimming connections and a watertight shorting cap
3. Not allow more than 2.5 percent of the rated lumens to project above 80 degrees measured up from the vertical plane in the direction of the roadway
4. Have equipment identification character labels outside the unit on the side that will face the road. Equipment identification characters consist of:
 - 4.1. R1 for Roadway 1, R2 for Roadway 2, R3 for Roadway 3, and R4 for Roadway 4
 - 4.2. Rated wattage

The luminaire's housing must have a slip fitter that must:

1. Fit on mast arms with outside diameters from 1-5/8 to 2-3/8 inches
2. Be adjustable to a minimum of ± 5 degrees from the axis of the tenon in a minimum of 5 steps: +5, +2.5, 0, -2.5, -5
3. Have clamping brackets that:
 - 3.1. Are made of corrosion-resistant materials or treated to prevent galvanic reactions
 - 3.2. Do not bottom out on the housing bosses when adjusted within the designed angular range
 - 3.3. Do not permanently set in excess of 1/32 inch when tightened

86-1.02K(3) Overhead Sign Luminaires

An overhead sign luminaire must:

1. Have a uniformity average to minimum ratio of 10:1 for the distribution of light reflected on a 16' wide by 10' high sign panel
2. Not allow more than 2.5 percent of the rated lumens to project above 65 degrees measured up from the horizontal plane in the direction of the sign panel
3. Mount at a maximum height of 12 inches above the top of the mounting rails
4. Mount directly to the sign structure as shown or with a mounting adapter that meets the material requirements of the luminaire's housing

Replace section 86-1.02M with:

10-19-18

86-1.02M Photoelectric Controls

Photoelectric control types are as shown in the following table:

Photoelectric Control Types

Control type	Description
I	Pole-mounted photoelectric unit. Test switch and a 15-A circuit breaker per ungrounded conductor, housed in an enclosure.
II	Pole-mounted photoelectric unit. Contactor, a 15-A circuit breaker per ungrounded conductor, and test switch located in a service equipment enclosure.
III	Pole-mounted photoelectric unit. Contactor, a 15-A circuit breaker per ungrounded conductor, and a test switch housed in an enclosure.
IV	A photoelectric unit that plugs into a NEMA twist-lock receptacle, integral with the luminaire.
V	A photoelectric unit, contactor, a 15-A circuit breaker per ungrounded conductor, and test switch located in a service equipment enclosure.

The pole-mounted adaptor for Type I, II, and III photoelectric controls must include a terminal block and cable supports or clamps to support the wires.

Photoelectric unit must:

1. Have a screen to prevent artificial light from causing cycling.
2. Have a rating of 60 Hz, 105-130 V(ac), 210-240 V(ac), or 105-240 V(ac).

3. Operate at a temperature range from -20 to 55 degrees C.
4. Consume less than 10 W.
5. Be a 3-prong, twist-lock type with a NEMA IP 65 rating, ANSI C136.10-compliant.
6. Have a fail-on state.
7. Fit into a NEMA-type receptacle.
8. Turn on from 1 to 5 footcandles and turn off from 1.5 to 5 times the turn-on level. Measurements must be made by procedures in *EEI-NEMA Standards for Physical and Electrical Interchangeability of Light-Sensitive Control Devices Used in the Control of Roadway Lighting*.

Type I, II, III, and V photoelectric controls must have a test switch to allow manual operation of the lighting circuit. Switch must be:

1. Single-hole mounting, toggle type
2. 15 A, single pole and single throw
3. Labeled *Auto-Test* on a nameplate

Photoelectric control's contactor must be:

1. Normally open
2. Mechanical-armature type with contacts of fine silver, silver alloy, or equal or better material
3. Installed to provide a minimum space of 2-1/2 inches between the contactor terminals and the enclosure's sides

The terminal blocks must be rated at 25 A, 600 V(ac), molded from phenolic or nylon material, and be the barrier type with plated-brass screw terminals and integral marking strips.

Replace section 86-1.02N with:

10-19-18

86-1.02N Fused Splice Connectors

The fused splice connector for 240 and 480 V(ac) circuits must simultaneously disconnect both ungrounded conductors. The connector must not have exposed metal parts except for the head of the stainless steel assembly screw. The head of the assembly screw must be recessed a minimum of 1/32 inch below the top of the plastic boss that surrounds the head.

The connector must protect the fuse from water or weather damage. Contact between the fuse and fuse holder must be spring loaded.

Fuses must:

1. Be standard, midget, ferrule type
2. Have a nontime-delay feature
3. Be 13/32 by 1-1/2 inches

Fuse ratings for luminaires are shown in the following table:

Fuse Current Rating Requirements		
Circuit voltage	Fuse voltage rating	Soffit and roadway luminaires
120 V(ac)	250 V(ac)	5 A
240 V(ac)	250 V(ac)	5 A
480 V(ac)	500-600 V(ac)	5 A

Fuse ratings for transformers are shown in the following table:

Fuse Current Rating Requirements

Circuit voltage	Fuse voltage rating	Fuse current rating for Transformers (primary side)		
		Single phase (two wires)	2 kVA	3 kVA
120 V(ac)	250 V(ac)	10 A	20 A	30 A
240 V(ac)	250 V(ac)	6 A	10 A	20 A
480 V(ac)	500-600 V(ac)	3 A	6 A	10 A

Replace section 86-1.02P(1) with:

10-19-18

86-1.02P(1) General

The enclosures must be rated NEMA 3R and include a dead front panel and a hasp with a 7/16-inch-diameter hole for a padlock.

Except for a service equipment enclosure, an enclosure must:

1. Be manufactured from steel and either galvanized, cadmium plated, or powder coated
2. Mount to a standard, pole, post, or sign structural frame
3. Provide a minimum space of 2-1/2 inches between the internal components and the enclosure's sides

The enclosure's machine screws and bolts must not protrude outside the cabinet wall.

The fasteners on the exterior of an enclosure must be vandal resistant and not be removable. The exterior screws, nuts, bolts, and washers must be stainless steel.

Replace the 1st paragraph of section 86-1.02P(2) with:

04-19-19

Service equipment enclosure must:

1. Comply with the Electric Utility Service Equipment Requirements Committee
2. Meet the requirements of the service utility
3. Be watertight
4. Be factory wired and manufactured from steel and galvanized or have factory-applied, rust-resistant prime and finish coats, except Types II and III
5. Be marked as specified in NEC to warn of potential electric-arc flash hazards

Delete the 5th paragraph of 86-1.02P(2).

04-19-19

Add between 6th and 7th paragraphs of section 86-1.02P(2):

10-19-18

Service equipment enclosure must have the meter view windows located on the front side of the enclosure for Types III-AF, BF, CF and DF.

Service equipment enclosure must have the meter view windows located on the back side of the enclosure for Types III-AR, BR, CR and DR.

Replace the 7th paragraph of section 86-1.02P(2) with:

04-19-19

The meter area must have a sealable, lockable, weather-tight cover that can be removed without the use of tools.

Delete the 2nd sentence of the 9th paragraph of section 86-1.02P(2).

04-19-19

Delete section 86-1.02P(3).

10-19-18

Replace section 86-1.02Q(4)(a) with:

10-19-18

86-1.02Q(4)(a) General

The doors of a telephone demarcation cabinet must be attached using continuous aluminum steel piano hinges.

Add between the 2nd and 3rd paragraphs of section 86-1.02R(2):

10-19-18

Bracket arms must be long enough to allow proper alignment of signals and backplate installation.

Replace item 2 in the list in the 5th paragraph of section 86-1.02R(4)(a)(iii) with:

10-19-18

2. Be a black color throughout, including the door, matching color no. 17038, 27038, or 37038 of AMS-STD-595

Add to the beginning of section 86-1.02T:

04-19-19

Accessible pedestrian signal must be on the Authorized Material List for Accessible Pedestrian Signals.

Replace the 5th and 6th paragraphs of section 86-1.02T with:

10-19-18

The color of a metallic housing must match color no. 33538 of AMS-STD-595.

The color of a plastic housing must match color no. 17038, 27038, or 37038 of AMS-STD-595.

Replace the 7th paragraph of section 86-1.02T with:

04-19-19

Accessible pedestrian signal must:

1. Have controllable and programmable volume level and messaging
2. Be weatherproof and shockproof

Replace the 11th paragraph of section 86-1.02T with:

10-19-18

The cable between the accessible pedestrian signal assembly and the pedestrian signal head must be rated for outdoor use and have a:

1. Minimum four no. 18 stranded or larger tinned copper conductors with a minimum insulation thickness of 15 mils
2. Cable jacket with a minimum thickness of 20 mils and rated for a minimum:
 - 2.1. 300 V(ac)

- 2.2. 80 degrees C
- 3. Nominal outside diameter less than 350 mils
- 4. Conductor color code of black, white, red and green

Replace the 1st paragraph of section 86-1.02U with:

10-19-18

The housing for a push button assembly must be made of die-cast aluminum, permanent mold-cast aluminum, or UV-stabilized self-extinguishing structural plastic.

The housing must have a uniform color that matches color no. 17038, 27038, or 37038 of AMS-STD-595.

Replace the 2nd paragraph of section 86-1.02W(4) with:

10-19-18

The cured hot-melt rubberized asphalt sealant must comply with the requirements shown in the following table:

Cured Hot-Melt Rubberized Asphalt Sealant Requirements

Quality characteristic	Test method	Requirement
Cone penetration, 25 °C, 150 g, 5 s (max, 1/10 mm)	ASTM D5329	35
Flow, 60 °C, 5 hr (max, mm)		5
Resilience, 25 °C (min, %)		25
Softening point (min, °C)	ASTM D36	82
Ductility, 25 °C, 5 cm/min (min, cm)	ASTM D113	30
Flash point, Cleveland Open Cup (min, °C)	ASTM D92	288
Viscosity, no. 27 spindle, 20 rpm, 190 °C (Pa•s)	ASTM D4402	2.5–3.5

Replace the 2nd paragraph of section 86-1.02Y with:

10-19-18

A transformer must be a dry type designed for operation on a 60 Hz supply. The transformer must have a decal showing a connection diagram. The diagram must show either color coding or wire tagging with primary (H1, H2) or secondary (X1, X2) markers and the primary and secondary voltage and volt-ampere rating. A transformer must comply with the electrical requirements shown in the following table:

Transformer Electrical Requirements

Quality characteristic	Requirement
Rating (V(ac))	120/240, 120/480, 240/120, 240/480, 480/120, or 480/240
Efficiency (%)	> 95
Secondary voltage regulation and tolerance from half load to full load (%)	±3

AA

87 ELECTRICAL SYSTEMS

04-19-19

Replace *Reserved* in section 87-1.01C with:

10-19-18

Submit a digital file for geographic information system mapping for:

1. Conduit
2. Pull boxes
3. Cabinets
4. Service equipment enclosures
5. Standards

The digital file must consist of:

1. Longitudinal and latitude coordinates, under the WGS84 reference coordinate system. The coordinates must be in decimal format having 6 significant figures after the decimal point. Coordinates must be read at the center of pull boxes, cabinet, standards, and service equipment enclosures; and on top of conduit at 20-foot intervals before backfill.
2. Type, depth and size for conduits.
3. Type for pull boxes, standards, cabinets, and service equipment enclosures.

Replace item 4 in the list in the 1st paragraph of section 87-1.01D(2)(a) with:

4. Luminaires

10-19-18

Replace section 87-1.01D(2)(d) with:

10-19-18

87-1.01D(2)(d) Piezoelectric Axle Sensors

Piezoelectric axle sensors test consists of:

1. Demonstrating for each sensor:
 - 1.1. Capacitance is within 20 percent of the value shown on the sensor's data sheet
 - 1.2. Dissipation factor is less than 0.04 nF when measured in the 20 nF range
 - 1.3. Resistance is greater than 20 Megaohms
2. Collecting a minimum of 100 vehicle records for each lane and demonstrating:
 - 2.1. Volume is within ± 3 percent accuracy
 - 2.2. Vehicle classification is within 95 percent accuracy by type

Replace the 7th paragraph of section 87-1.03A with:

10-19-18

Notify the Engineer immediately if an existing facility is damaged by your activities:

1. Damaged existing traffic signal systems must be repaired or replaced within 24 hours. If the system cannot be fixed within 24 hours or it is located on a structure, provide a temporary system until the system can be fixed.
2. Damaged existing lighting systems must be repaired or replaced by nightfall. If the system cannot be fixed by nightfall, provide a temporary system until the system can be fixed.

Add to the end of section 87-1.03A:

10-19-18

Collect the geographic information system mapping data.

Replace the 12th paragraph of section 87-1.03B(1) with:

10-19-18

For Type 1, 2, and 5 conduits, use threaded bushings and bond them using a jumper. For other types of conduit, use nonmetallic bushings or end bell.

Replace the 3rd paragraph of section 87-1.03B(3)(a) with:

10-19-18

Place a minimum of 2 inches of sand bedding in a trench before installing the conduit and 18 inches of slurry cement over the conduit before placing additional backfill material.

The slurry must be pigmented to match AMS-STD-595.

Replace the 1st sentence in the 6th paragraph of section 87-1.03B(3)(c) with:

10-19-18

Backfill trench with slurry concrete under section 19-3.02E.

Replace the 9th paragraph of section 87-1.03B(3)(c) with:

10-19-18

Install innerducts as one continuous unit between vaults. Innerducts may be interrupted inside pull boxes located between vaults and cabinets.

Replace section 87-1.03D with:

10-19-18

87-1.03D Reserved

Replace section 87-1.03E(2) with:

04-19-19

Dig a trench for the electrical conduits or direct burial cables. Do not excavate until the installation of the conduit or direct burial cables.

Place excavated material in a location that will not interfere with traffic or surface drainage.

After placing the conduit or direct burial cable, backfill the trench.

Compact the backfill to a minimum relative compaction of:

1. 95 percent when placed within the hinge points and in areas where pavement is to be constructed
2. 90 percent when placed outside the hinge points and not under pavement

Restore the sidewalks, pavement, and landscaping at a location before starting excavation at another location.

Replace section 87-1.03E(3) with:

10-19-18

87-1.03E(3) Concrete Pads, Foundations, and Pedestals

Construct foundations for standards, poles, metal pedestals, and posts under section 56-3.

Construct concrete pads, foundations, and pedestals for controller cabinets, telephone demarcation cabinets, and service equipment enclosures on firm ground.

Install anchor bolts using a template to provide proper spacing and alignment. Moisten the forms and ground before placing the concrete. Keep the forms in place until the concrete sets for at least 24 hours to prevent damage to the surface.

Use minor concrete for pads, foundations, and pedestals.

Construct a pad in front of a Type III service equipment enclosure. The pad must be 24 inches in length, 4 inches in thickness, and must match the width of the foundation.

In unpaved areas, place the top of the foundation 6 inches above the surrounding grade, except place the top:

1. 1 foot 6 inches above the grade for 336L cabinets
2. 1 foot 8 inches above the grade for Type C telephone demarcation cabinets
3. 2 inches above the grade for Type III service equipment enclosures

The pad must be 2 inches above the surrounding grade in unpaved areas.

In and adjacent to the sidewalk and other paved areas, place the top of the foundation 4 inches above the surrounding grade, except place the top:

1. 1 foot 6 inches above the grade for 336L cabinets
2. 1 foot 8 inches above the grade for Type C telephone demarcation cabinets
3. Level with the finished grade for Type G and Type A cabinets and Type III service equipment enclosures

The pad must be level with the finished grade in paved areas.

Apply an ordinary surface finish under section 51-1.03F.

Allow the foundation to cure for at least 7 days before installing any equipment.

Replace the last paragraph of section 87-1.03F(1) with:

Install a tracer wire.

04-19-19

Replace the 1st paragraph of section 87-1.03F(3)(c)(ii) with:

Install a Type 1 or 2 inductive loop conductor except use Type 2 for Type E and F loop detectors.

10-19-18

Delete the last paragraph of section 87-1.03G.

10-19-18

Replace the 4th paragraph of section 87-1.03H(2) with:

Use Method B as follows:

1. Cover the splice area completely with an electrical insulating coating and allow it to dry.
2. Apply 3 layers of half-lapped, PVC electrical tape.
3. Apply 2 layers of butyl-rubber, stretchable tape with liner.
4. Apply 3 layers of half-lapped, PVC, pressure-sensitive, adhesive tape.
5. Cover the entire splice with an electrical insulating coating and allow it to dry.

10-19-18

Replace section 87-1.03N with:

87-1.03N Fused Splice Connectors

Install a fuse splice connector with a fuse in each ungrounded conductor for luminaires, except for overhead sign luminaires. The connector must be located in the pull box adjacent to the luminaires.

10-19-18

If the pull box for the roadway luminaire is tamper resistant, install a fuse splice connector with 10 A fuse in the pull box and an additional fuse splice connector with a 5 A fuse in the handhole.

Install a fuse splice connector with a fuse on primary side of transformer.

Crimp the connector terminals onto the ungrounded conductors using a tool under the manufacturer's instructions. Insulate the terminals and make them watertight.

Add to the end of section 87-1.03T:

10-19-18

When replacing an existing accessible pedestrian signal, the housing color must match the color of the existing housing.

Add to the end of section 87-1.03U:

10-19-18

When replacing an existing push button assembly, the housing color must match the color of the existing housing.

Add between the 1st and 2nd paragraphs of section 87-1.03Y:

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Use a submersible type transformer inside pull boxes.

Replace the 2nd paragraph of section 87-2.03A with:

10-19-18

Tighten the cap screws of the luminaire's clamping bracket to 10 ft-lb for roadway luminaires.

Replace section 87-3 with:

10-19-18

87-3 SIGN ILLUMINATION SYSTEMS

87-3.01 GENERAL

Section 87-3 includes specifications for constructing sign illumination systems.

Sign illumination system includes:

1. Foundations
2. Pull boxes
3. Conduit
4. Conductors
5. Overhead sign luminaires
6. Service equipment enclosure
7. Photoelectric control

The components of a sign illumination system are shown on the project plans.

87-3.02 MATERIALS

Reserved

87-3.03 CONSTRUCTION

Perform the conductor test.

Install overhead sign luminaires under the manufacturer's instructions.

Do not modify the sign structure or mounting channels.

Perform the operational tests for the system.

87-3.04 PAYMENT

Not Used

Replace section 87-4.01D with:

10-19-18

87-4.01D Quality Assurance

Reserved

Replace section 87-4.02B with:

10-19-18

87-4.02B Battery Backup System

A battery backup system includes the cabinet, batteries, and the Department-furnished electronics assembly.

The electronics assembly includes the inverter/charger unit, power transfer relay, manually-operated bypass switch, battery harness, utility interconnect wires, battery temperature probe, and relay contact wires.

Replace the 2nd sentence in the 15th paragraph of section 87-4.02C with:

10-19-18

The background must comply with color no. 14109 of AMS-STD-595.

Replace section 87-4.03B with:

10-19-18

87-4.03B Battery Backup System Cabinets

Install the battery backup system cabinet to the right of the controller cabinet.

If installation on the right side is not possible, obtain authorization for installation on the left side.

Provide access for power conductors between the cabinets using:

1. 2-inch nylon-insulated, steel chase nipple
2. 2-inch steel sealing locknut
3. 2-inch nylon-insulated, steel bushing

Remove the jumper between the terminals labeled *BBS-1* and *BBS-2* in the 5 position terminal block in the controller cabinet before connecting the Department-furnished electronics assembly.

Replace section 87-7.02 with:

10-19-18

87-7.02 MATERIALS

Flashing beacon control assembly includes:

1. Enclosure.
2. Barrier-type terminal blocks rated for 25 A, 600 V(ac), made of molded phenolic or nylon material and have plated-brass screw terminals and integral marking strips.
3. Solid state flasher complying with section 8 of NEMA standards publication no. TS 1 for 10 A, dual circuits.

4. 15-A, circuit breaker per ungrounded conductor.
5. Single-hole-mounting toggle type, single-pole, single-throw switches rated at 12-A, 120 V(ac). Switches must be furnished with an indicating nameplate reading *Auto - Test*. A 15-A circuit breaker may be used in place of the toggle switch.

Replace 87-8 with:

10-19-18

87-8 PEDESTRIAN HYBRID BEACON SYSTEMS

87-8.01 GENERAL

87-8.01A Summary

Section 87-8 includes specifications for constructing pedestrian hybrid beacon system.

A pedestrian hybrid beacon system includes:

1. Foundations
2. Pull boxes
3. Conduit
4. Conductors and cables
5. Standards
6. Pedestrian hybrid beacon face
7. Pedestrian signal heads
8. Service equipment enclosure
9. Department-furnished controller assembly
10. Accessible pedestrian signals
11. Push button assemblies
12. Luminaires
13. Fuse splice connectors
14. Battery backup system

The components of a pedestrian hybrid beacon system are shown on the project plans.

87-8.01B Definitions

Reserved

87-8.01C Submittals

Reserved

87-8.01D Quality Assurance

87-8.01D(1) General

Reserved

87-8.01D(2) Quality Control

Verify the sequence for the pedestrian hybrid beacon system per California Chapter 4F, Figure 3F-3 "Sequence for a Pedestrian Hybrid Beacon" during the operational test.

Test the battery backup system under section 87-1.01D(2)(c).

87-8.02 MATERIALS

87-8.02A General

The system must comply with California *MUTCD*, Chapter 4F.

The battery backup system must comply with section 87-4.02B.

87-8.02B Pedestrian Hybrid Beacon Face

A pedestrian hybrid beacon face consists of three 12-inch signal heads.

87-8.03 CONSTRUCTION

Install pedestrian hybrid beacon system under sections 87-4.03A and 87-4.03B.

87-8.04 PAYMENT

Not Used

Replace the 1st paragraph of section 87-12.03 with:

Install changeable message sign on sign structure under section 56-2.

10-19-18

Replace section 87-14.02 with:

87-14.02 MATERIALS

87-14.02A General

Vehicle speed feedback sign consists of a housing, display window, and radar unit.

Sign must:

1. Comply with the California MUTCD, Chapter 2B
2. Have an operating voltage of 120 V(ac) for permanent installations
3. Have a maximum weight of 45 lb
4. Have a wind load rating of 90 mph
5. Have an operating temperature range from -34 to 165 degrees F
6. Have a retroreflective white sheeting background

10-19-18

87-14.02B Housings

Housing must:

1. Be weatherproof (NEMA 3R or better) and vandal resistant
2. Be made of 0.09-inch-gauge welded aluminum with the outer surfaces being UV resistant
3. Have the manufacturer's name, model number, serial number, date of manufacture, rated voltage and rated current marked inside
4. Have the internal components easily accessible for field repair without removal of the sign

87-14.02C Display Windows

Display window consists of a cover, LED character display, and dimming control. Character display and cover must deflect together without damage to the internal electronics and speed detection components.

Cover must be:

1. Vandal resistant and shock absorbent
2. Field replaceable with the removal of external stainless-steel, tamper proof fasteners

Cover must be made of a minimum 0.25-inch-thick, shatter-resistant polycarbonate.

LED character display must:

1. Consist of two 7-segment, solid-state, numeric characters, which must:
 - 1.1. Be a minimum 15 inches in height
 - 1.2. Be visible and legible from a minimum distance of 1500 feet and legible from a minimum distance of 750 feet
 - 1.3. Consist of a minimum 16 LEDs, which must:
 - 1.3.1. Be amber and have a wavelength from 590 to 600 nm and rated for minimum 100,000 hours
 - 1.3.2. Must maintain a minimum 85 percent of the initial light output after 48 months of continuous use over the temperature range
2. Be capable of displaying the detected vehicle speed within 1 second

3. Remain blank when no vehicles are detected within the radar detection zone
4. Have the option to flash the pre-set speed limit when the detected vehicle speed is 5 miles higher than the pre-set speed
5. Be viewable only by the approaching traffic

Dimming control must:

1. Automatically adjust the character light intensity to provide optimum character visibility and legibility under all ambient lighting conditions
2. Have minimum 3 manual dimming modes of different intensities

87-14.02D Radar Units

Radar unit must:

1. Be able to detect up to 3 lanes of approaching traffic
2. Operate with an internal, low power, 24.159 GHz (K-band)
3. Be FCC approved Part 15 certified
4. Have a speed accuracy of ± 1 mph
5. Have a maximum 15 W power consumption

Replace 87-19 with:

10-19-18

87-19 FIBER OPTIC CABLE SYSTEMS

87-19.01 GENERAL

87-19.01A Summary

Section 87-19 includes specifications for constructing fiber optic cable systems.

A fiber optic cable system includes:

1. Conduit and accessories
2. Vaults
3. Warning tape
4. Fiber optic cables
5. Fiber optic splice enclosures
6. Fiber distribution units
7. Fiber optic markers
8. Fiber optic connectors and couplers

The components of a fiber optic system are shown on the project plans.

87-19.01B Definitions

Reserved

87-19.01C Submittals

At least 15 days before cable installation, submit:

1. Manufacturer's procedures for pulling fiber optic cable
2. Test reports from a laboratory accredited to International Standards Organization/International Electrotechnical Commission 17025 by the American Association for Laboratory Accreditation (A2LA) or the ANSI-ASQ National Accreditation Board (ANAB) for:
 - 2.1. Water penetration
 - 2.2. Cable temperature cycling
 - 2.3. Cable impact
 - 2.4. Cable tensile loading and fiber strain
 - 2.5. Cable compressive loading
 - 2.6. Compound flow
 - 2.7. Cyclic flexing
3. Proof of calibration for the test equipment including:

- 3.1. Name of calibration facility
- 3.2. Date of calibration
- 3.3. Type of equipment, model number and serial number
- 3.4. Calibration result

Submit optical time-domain reflectometer data files for each test in a Microsoft Excel format.

After performing the optical time-domain reflectometer test and the power meter and light source test, submit within 4 business days a hard copy and electronic format:

1. Cable Verification Worksheet
2. Segment Verification Worksheet
3. Link Loss Budget Worksheet

The worksheets are available at the Division of Construction website.

87-19.01D Quality Assurance

87-19.01D(1) General

Reserved

87-19.01D(2) Quality Control

Notify the Engineer 4 business days before performing field tests. Include exact location of the system or components to be tested. Do not proceed with the testing until authorized. Perform each test in the presence of the Engineer.

The optical time-domain reflectometer test consists of:

1. Inspecting the cable segment for physical damage.
2. Measuring the attenuation levels for wavelengths of 1310 and 1550 nm in both directions for each fiber using the optical time-domain reflectometer.
3. Comparing the test results with the data sheet provided with the shipment. If there are attenuation deviations greater than 5 percent, the test will be considered unsatisfactory and the cable segment will be rejected. The failure of any single fiber is a cause for rejection of the entire segment. Replace any rejected cable segments and repeat the test.

The power meter and light source test consists of:

1. Testing each fiber in a link using a light source at one end of the link and a power meter at the other end
2. Measuring and recording the power loss for wavelengths of 1310 and 1550 nm in both directions

Index matching gel is not allowed.

Installation and splicing of the fiber optic cable system must be performed by a certified fiber optic installer.

The optical time-domain reflectometer test and the power meter and light source test must be performed by a certified fiber optic technician.

The certification for the fiber optic installer and fiber optic technician must be from an organization recognized by the International Certification Accreditations Council and must be current throughout the duration of the project.

87-19.02 MATERIALS

87-19.02A General

All metal components of the fiber optic cable system must be corrosion resistant.

All connectors must be factory-installed and tested.

Patch cords, pigtailed, and connectors must comply with ANSI/TIA-568.

Pigtails must have a minimum 80 N pull out strength.

A splice cassette may be used in place of a pigtail and a splice tray.

Each cable reel must have a weatherproof label or tag with information specified in ANSI/ICEA S-87-640 including:

1. Contractor's name
2. Contract number
3. Number of fibers
4. Cable attenuation loss per fiber at 1310 and 1550 nm

The labeled or tagged information must also be in a shipping record in a weatherproof envelope. The envelope must be removed only by the Engineer.

87-19.02B Vaults

A vault must:

1. Comply with section 86-1.02C and AASHTO HS 20-44, and load tested under AASHTO M 306.
2. Be a minimum:
 - 2.1. 4 feet wide by 4 feet high by 4 feet long nominal inside dimensions for box type.
 - 2.2. 4 feet high by 4 feet outside diameter for round type.
3. Have a minimum access of:
 - 3.1. 30 inches diameter for round type.
 - 3.2. 3 feet wide by 3 feet long for box type.
4. Be precast either modular or monolithic.
5. Have cable racks installed on the interior sides. A rack must:
 - 5.1. Be fabricated from ASTM A36 steel plate.
 - 5.2. Support a minimum of 100 pounds per rack arm.
 - 5.3. Support a minimum of 4 splice enclosures and a minimum of 4 cables with a minimum slack of 50 feet each.
 - 5.4. Be hot-dip galvanized after manufacturing.
 - 5.5. Be bonded and grounded.
6. Have a minimum:
 - 6.1. Two 4-inch diameter knockouts on each side for box type.
 - 6.2. Two 4-inch diameter knockouts placed every 90 degrees for round type.
7. Have a minimum 2-inch-diameter drain hole at the center of base.

Entry points for knockouts must not cause the cable to exceed its maximum bend radius.

The access cover must:

1. Be a two-piece torsion-assisted sections or a minimum 30-inch-diameter cast iron.
2. Have inset lifting pull slots.
3. Have markings *CALTRANS* and *FIBER OPTIC*.

87-19.02C Fiber Optic Cable

The fiber optic cable must:

1. Comply with 7 CFR parts 1755.900, 1755.901, and 1755.902, and ANSI/ICEA S-87-640
2. Be a singlemode, zero-dispersion, and have non-gel loose type buffer tubes
3. Have no splices
4. Have a Type H or Type M outer jacket
5. Be shipped on a reel
6. Have 10 feet of length on each end of the cable accessible for testing

87-19.02D Fiber Optic Splice Enclosures

A fiber optic splice enclosure must:

1. Not exceed 36 inches in length, 8 inches in width, and 8 inches in height
2. Be made of thermoplastic material, weather proof, chemical and UV resistant, and re-sealable
3. Accommodate a minimum of 8 internal splice trays
4. Have from 1/4 to 1 inch in diameter cable entry ports

5. Have brackets, clips and cable ties
6. Have means to anchor the dielectric member of the fiber optic cable
7. Include grounding hardware

87-19.02E Fiber Distribution Units

The fiber distribution unit consists of a housing, a patch panel, a 12-multicolor pigtail, and a splice tray.

The fiber distribution unit must be self-contained and pre-assembled.

The housing must:

1. Be a 19-inch rack-mountable modular-metal enclosure
2. Be a one rack unit
3. Have cable clamps to secure buffer tube to the chassis
4. Have cable accesses with rubber grommets or similar material to prevent the cable from coming in contact with the bare metal
5. Be weatherproof
6. Have a hinged top door with a latch or thumbscrew to hold it in the closed position

A patch panel must have a minimum of 12-singlefiber type connector sleeves.

A pigtail must:

1. Be a simplex single mode fiber in a 900 μm tight buffer with a 12-inch-outer-diameter PVC jacket
2. Have a fiber optic connector attached on one end and bare fiber on the other end
3. Be at least 3 feet in length
4. Have the manufacturer's part number on the jacket

Pigtails must be single-fiber or ribbon type.

87-19.02F Patch Cords

Patch cords must:

1. Be a singlemode fiber in a 900 μm tight buffer with a 0.12-inch-outer-diameter PVC jacket
2. Have fiber optic connectors attached on both ends
3. Be at least 6 feet in length
4. Have manufacturer's part number on the jacket

Duplex patch cords must be of round cable structure, and not have zip-cord structure.

87-19.02G Splice Trays

Splice trays must:

1. Have brackets to spool incoming fibers a minimum of 2 turns.
2. Have means to secure and protect incoming buffer tubes, pigtails, and a minimum of 12 heat shrink fusion splices.
3. Be stackable.
4. Have a snap-on or hinged cover. The cover may be transparent.

87-19.02H Fiber Optic Markers

Fiber optic markers must be:

1. Type K-2 (CA) object markers for vaults or pull boxes.
2. Disk markers for paved areas and transition points from unpaved to paved areas. The disk marker must be metallic, lead free and 4 inches in diameter, and must have a mounting stem at the center of the disk. The mounting stem must be a minimum 3 inches long and a minimum 0.70 inch in diameter.
3. Non-reflective Class 1, Type F, flexible post delineators for unpaved areas.

87-19.02I Fiber Optic Connectors and Couplers

Connectors must be:

1. 0.1-inch ceramic ferrule pre-radiused type
2. Capped when not used

Couplers must be made of the same material as the connector's housing and have ceramic sleeves.

Singlemode fiber optic connectors must have a yellow strain relief boot or a yellow base.

87-19.03 CONSTRUCTION

87-19.03A General

Perform the optical time-domain reflectometer test:

1. On the fiber optic cable upon its arrival to the job site and before its installation. Complete the Cable Verification Worksheet. Do not install the fiber optic cable until the Engineer's written approval is received.
2. After the fiber optic cable segments have been pulled, but before breakout and termination. Complete the Segment Verification Worksheet.
3. Once the passive cabling system has been installed and is ready for activation. If the measured individual fusion splice losses exceed -0.30 dB, re-splice and retest. At the conclusion of the optical time-domain reflectometer test, perform the power meter and light source test. If the measured link loss exceeds the calculated link loss, replace the unsatisfactory cable segments or splices and retest. Complete the Link Loss Budget Worksheet.

87-19.03B Vaults Installation

Install a vault as shown and with the side facing the roadway a minimum of 2 feet from the edge of pavement or back of dike, away from traffic.

Install the top of the vault flush with surrounding grade in paved areas and 2 inches above the surrounding grade in unpaved areas.

Place 6 inches of minor concrete around vaults. In unpaved areas, finish top of concrete at a 2 percent slope away from cover. In paved areas, finish top of concrete to match existing slope.

Bolt the steel cover to the vault when not working in it.

87-19.03C Fiber Optic Cable Installation

Install fiber optic cable by a certified installer or a representative from the fiber optic cable manufacturer during installation.

When using mechanical aids to install fiber optic cable:

1. Maintain a cable bend radius at least twenty times the outside diameter of the cable
2. Use cable grips having a ball bearing swivel
3. Use a pulling force on a cable not to exceed 500 pound-foot or manufacturer's recommended pulling tension, whichever is less

When installing the cable using the air blown method, the cable must withstand a static air pressure of 110 psi.

Lubricate the cable using a lubricant recommended by the cable manufacturer.

Install fiber optic cable without splices except where shown.

Provide a minimum of 65 feet of slack for each fiber optic cable at each vault. Divide the slack equally on each side of the splice enclosure.

Install tracer wires in the fiber optic conduits and innerducts as shown. Provide a minimum 5 feet of slack tracer wire in each pull box and vault from each direction. You may splice tracer wire at intervals of not less than 500 feet and only inside vaults or pull boxes.

If a fiber optic cable and tracer wire is installed in an innerduct, pulling a separate fiber optic cable into a spare duct to replace damaged fiber will not be allowed.

Apply a non-hygroscopic filling compound to fiber optic cable openings.

Seal the ends of conduit and innerducts after cables are installed.

Install strain relief for fiber optic cable entering a fiber optic enclosure.

Identify fibers and cables by direct labeling, metal tags, or bands fastened in such a way that they will not move. Use mechanical methods for labeling.

Provide identification on each fiber optic cable or each group of fiber optic cables in each vault and at the end of terminated fibers. Fiber optic cable must be identified as shown in the following table:

Cable Identification^a

Sequence order	Description	Code	Numbers of characters
1	Fiber type	S: Singlemode	1
2	Fiber count	###: Example 048	3
3	Begin point	T: TMC H: Hub V: Video Node D: Data Node C: Cable Node TV: Camera CM: CMS E: Traffic Signal RM: Ramp Meter TM: Traffic Monitoring/ Count Station/Vehicle Count Station (VDS, TMS) HA: Highway Advisory Radio EM: Extinguishable Message Sign RW: Roadway Weather Information System WM: Weigh In Motion WS: Weigh-Station Bypass System SV: Vault SC: Splice Cabinet	1 or 2
4	Begin point county abbreviation	AA or AAA: Examples: Orange (ORA), San Mateo (SM)	2 or 3
5	Begin point route number	###: Examples: 005, 082, 114	3
6	Begin point post mile	#####: 02470 (example 024.70): Actual PM value to the 1/100 value	5
7	End Point	In the same way as for Begin Point	1 or 2
8	End point county abbreviation	In the same way as for Begin Point County Abbreviation	2 or 3
9	End point route number	In the same way as Begin Point Route Number	3
10	End point post mile	In the same way as Begin Point Post Mile	5

^aCable identification example: The cable code S 048 SV SM 084 02470 SV SC 082 02510 describes a singlemode, 48 strand, cable starting at a fiber optic vault in San Mateo County on Route 84 at post mile 24.70, and ending at another fiber optic vault in Santa Clara County on Route 82 at post mile 25.10.

Place labels on the cables at the following points:

1. Fiber optic vault and pull box entrances and exits
2. Splice enclosures entrance and exit

3. Fiber distribution unit entrance

Lace fiber optic cable inside controller cabinets and secure to the cage.

Support the fiber optic cable within 6 inches from a termination and every 2 feet.

Secure fiber optic cables to the cable racks. Store excess cable in a figure 8 fashion.

87-19.03D Fiber Optic Cable Splices

Use fusion splicing for fiber optic cables.

Splice single-buffer tube cable to multi-buffer tube cable using the mid-span access method under manufacturer's instructions. Any mid-span access splice or fiber distribution unit termination must involve only those fibers being spliced as shown.

Place fiber splices in the splice enclosures installed in the vaults.

87-19.03E Splice Enclosures Installation

Maintain an equal amount of slack on each side of the splice enclosure.

Secure the fiber optic splices in splice tray.

Secure the splice trays to the inner enclosure.

Label cables and buffer tubes.

Do not seal fiber splice enclosure until authorized and the power meter and light source test is performed. Seal the enclosure under manufacturer's instructions.

Flash test the outer enclosure under manufacturer's instructions in the presence of the Engineer. Visually inspect the enclosure. If bubbles are present, identify the locations where the bubbles are present, take corrective actions and repeat the flash test until no bubbles are present.

Attach the splice enclosure to the side wall of a vault or hub with a minimum 2 feet distance between the ground and the bottom of the enclosure.

Secure fiber optic cables to the chassis using cable clamps for fiber optic units.

Connect a minimum of one bonding conductor to a grounding electrode after mounting the fiber optic enclosure to the wall. If there are multiple bonding conductors, organize the conductors in a neat way.

87-19.03F Fiber Optic Distribution Unit Installation

Spool incoming buffer tubes 2 feet in the splice tray and expose 1 foot of individual fibers.

Maintain a minimum 2-inch-bend radius during and after installation in the splice tray.

Splice incoming fibers in the splice tray.

Restrain each fiber in the splice tray. Do not apply stress on the fiber when located in its final position.

Secure buffer tubes near the entrance of the splice tray.

Secure splice trays under manufacturer's instructions.

Label splice tray after splicing is completed.

Install patch cords in fiber distribution units and patch panels. Permanently label each cord and each connector in the panel with the system as shown.

87-19.03G Fiber Optic Markers Installation

Install fiber optic markers at 12-inch offset on the side furthest away from the edge of travel way:

1. For fiber optic cable at 500 feet apart in areas where the distance between vaults or pull boxes is greater than 500 feet
2. Adjacent to vaults and pull boxes

3. For fiber optic cable turns at:
 - 3.1. Beginning of the turn
 - 3.2. Middle of the arc
 - 3.3. End of the turn

When a fiber optic cable crosses a roadway or ramp, install a disk marker over the conduit trench on:

1. Every shoulder within 6 inches from the edge of pavement
2. Delineated median
3. Each side of a barrier

Install markers under section 81 except each retroreflective face must be parallel to the road centerline and facing away from traffic.

87-19.04 PAYMENT

Not Used

Replace section 87-20 with:

04-19-19

87-20.01 GENERAL

Section 87-20 includes specifications for providing, maintaining, and removing temporary electrical systems.

Obtain the Department's authorization for the type of temporary electrical system and its installation method.

A temporary system must operate on a continuous, 24-hour basis.

A temporary electrical system must have a primary power source and a back-up power source from:

1. Commercial power from a utility company
2. Generator system
3. Photovoltaic system

87-20.02 MATERIALS

87-20.02A General

Material and equipment may be new or used.

Temporary wood poles must comply with section 48-6.

The components of a temporary system are shown on the project plans.

If you use Type UF-B cable, the minimum conductor size must be no. 12.

A back-up power source must:

1. Have an automatic transfer switch
2. Start automatically and transfer the system load upon reaching the operating voltage in the event of a power source failure

87-20.02B Temporary Flashing Beacon Systems

A temporary flashing beacon system consists of a flashing beacon system, wood post, and a power source.

The system must comply with the specifications for a flashing beacon system in section 87-7, except it may be mounted on a wood post or a trailer.

87-20.02C Temporary Lighting Systems

A temporary lighting system consists of a lighting system, a power source, and wood poles.

The system must comply with the specifications for a lighting system in section 87-2, except it may be mounted on a wood pole or a trailer.

87-20.02D Temporary Signal Systems

A temporary signal system consists of a signal and lighting system, wood poles and posts, and a power source.

The system must comply with the specifications for a signal and lighting system in section 87-4, except:

1. Signal heads may be mounted on a wood pole, mast arm, tether wire, or a trailer
2. Flashing beacons may be mounted on a wood post, or a trailer

87-20.02E Generators

A generator must:

1. Be 120 V(ac) or 120/240 V(ac), 60 Hz, 2.5 kW minimum, continuous-duty type
2. Be powered by a gasoline, LPG, or diesel engine operating at approximately 1,800 rpm with an automatic oil feed
3. Be equipped to provide automatic start-stop operation with a 12 V starting system
4. Have generator output circuits that have overcurrent protection with a maximum setting of 15 A
5. Have enough fuel storage to operate when it is unattended
6. Have a spark arrester complying with Pub Cont Code § 4442

87-20.02F Automatic Transfer Switches

An automatic transfer switch must provide:

1. Line voltage monitoring in the event of a power outage that signals the back-up power source to start
2. Start delay, adjustable from 0 to 6 seconds, to prevent starting if the power outage is only momentary and a stop delay, adjustable from 0 to 8 minutes, to allow the back-up power source to unload
3. Transfer delay from 0 to 120 seconds to allow the back-up power source to stabilize before connecting to the load and retransfer delay from 0 to 32 minutes to allow the line voltage to stabilize
4. Mechanical interlock to prevent an application of power to the load from both sources and to prevent backfeeding from the back-up power source to the primary power source

87-20.03 CONSTRUCTION

87-20.03A General

Provide electrical and telecommunication services for temporary systems. Do not use existing services unless authorized.

Provide power for the temporary electrical systems.

Commercial power must be 120 V(ac) or 120/240 V(ac) single phase. Make arrangements with the utility company for providing service. Protect the power source in a locked enclosure. Provide keys to all locks to the Engineer.

Install conductors and cables in a conduit, suspended from wood poles at least 25 feet above the roadway, or use direct burial conductors and cables.

You may saw slots across paved areas for burial conductors and cables.

Install conduit outside the paved area at a minimum of 12 inches below grade for Type 1 and 2 conduit and at a minimum of 18 inches below grade for Type 3 conduit.

Install direct burial conductors and cables outside the paved area at a minimum depth of 24 inches below grade.

Place the portions of the conductors installed on the face of wood poles in either Type 1, 2, or 3 conduit between the point 10 feet above grade at the pole and the pull box. The conduit between the pole and the pull box must be buried at a depth of at least 18 inches below grade.

Place conductors across structures in a Type 1, 2, or 3 conduit. Attach the conduit to the outside face of the railing.

Quality characteristic	Test method
Specific gravity and absorption of coarse aggregate	ASTM C127
Specific gravity and absorption of fine aggregate	ASTM C128
Durability index for fine aggregate	California Test 229
Soundness	California Test 214
Resistance to degradation	ASTM C131
Organic impurities	California Test 213
Chloride concentration of water for washing aggregates and mixing concrete	California Test 422
Sulfate concentration of water for washing aggregates and mixing concrete	California Test 417
Impurities in water for washing aggregates and mixing concrete	ASTM C191 or ASTM C266 and ASTM C109

Add to the end of section 90-1.01C(8):

04-19-19

For CIP structural concrete members, submit test results within 3 business days after completing each QC test. For submittal, go to:

<http://dime.dot.ca.gov/>

For CIP structural concrete members, include the following with the test results:

1. Contract number
2. Mix design number
3. Test sample identification number
4. Date and time of test
5. Batch plant
6. Batch number
7. Bridge number and description of element
8. Supporting data and calculations
9. Name, certification number, and signature of the QC tester

If additional compressive strength test results are needed for CIP structural concrete members to facilitate your schedule, submit a plot of the strength projection curve.

Add to section 90-1.01C:

04-19-19

90-1.01C(11) Quality Control Plan

Section 90-1.01C(11) applies to CIP structural concrete members.

Submit 3 copies of the QC plan for review.

Submit an amended QC plan or an addendum to the QC plan when there are any changes to:

1. Concrete plants
2. Testing laboratories
3. Plant certification or laboratory accreditation status
4. Tester or inspector qualification status
5. QC personnel
6. Procedures and equipment
7. Material sources
8. Material testing

Allow the Department 5 business days to review an amended QC plan or an addendum to the QC plan.

90-1.01C(12) Concrete Materials Quality Control Summary Report

Section 90-1.01C(12) applies to CIP structural concrete members.

During concrete production for CIP structural concrete members, submit a concrete materials QC summary report at least once a month. The report must include:

1. Inspection reports.
2. Test results.
3. Documentation of:
 - 3.1. Test result evaluation by the QC manager.
 - 3.2. Any discovered problems or deficiencies and the corrective actions taken.
 - 3.3. Any testing of repair work performed.
 - 3.4. Any deviations from the specifications or regular practices with explanation.
4. Certificate of compliance for the structural concrete material signed by the QC manager. The certificate must state that the information contained in the report is accurate, the minimum testing frequencies specified in section 90-1.01D(10)(d) are met, and the materials comply with the Contract.

Add to section 90-1.01D:

04-19-19

90-1.01D(7) Qualifications

Section 90-1.01D(7) applies to CIP structural concrete members.

QC laboratory testing personnel must have an ACI Concrete Laboratory Testing Technician, Level 1 certification or an ACI Aggregate Testing Technician, Level 2 certification, whichever certification includes the test being performed.

QC field testing personnel and field and plant inspection personnel must have an ACI Concrete Field Testing Technician, Grade I certification.

90-1.01D(8) Certifications

Section 90-1.01D(8) applies to CIP structural concrete members.

Each concrete plant used for CIP structural concrete members must:

1. Have a current certification for ready mixed concrete production facilities from the National Ready Mixed Concrete Association. Plant Certification Checklist and supporting documentation must be available upon request.
2. Be tested and authorized under the Department's *MPQP*.

Each QC testing laboratory must be an authorized laboratory with current accreditation from the AASHTO Accreditation Program for the tests performed.

90-1.01D(9) Preconstruction Meeting for CIP Structural Concrete

Section 90-1.01D(9) applies to CIP structural concrete members.

Before concrete placement, hold a meeting to discuss the requirements for structural concrete QC. The meeting attendees must include the Engineer, the QC manager, and at least 1 representative from each concrete plant performing CIP structural concrete activities for the Contract.

90-1.01D(10) Quality Control

90-1.01D(10)(a) General

Section 90-1.01D(10) applies to CIP structural concrete members.

Develop, implement, and maintain a QC program that includes inspection, sampling, and testing of structural concrete materials for CIP structural concrete members.

Perform all sampling, testing, and inspecting required to control the process and to demonstrate compliance with the Contract and the authorized QC plan.

Provide a QC field inspector at the concrete delivery point while placement activities are in progress.

Provide a testing laboratory and the testing personnel for QC testing.

The QC inspector and the QC manager must be fully authorized by the Contractor to reject material.

QC testers and inspectors must be your employees or must be hired by a subcontractor providing only QC services. QC testers and inspectors must not be employed or compensated by a subcontractor or by other persons or entities hired by subcontractors who will provide other services or materials for the project.

If lightweight concrete, RSC or SCC is used as structural concrete, you must also comply with the sampling and testing specifications of that section.

90-1.01D(10)(b) Quality Control Plan

The QC plan must detail the methods used to ensure the quality of the work and provide the controls to produce concrete. The QC plan must include:

1. Names and documentation of certification or accreditation of the concrete plants and testing laboratories to be used
2. Names, qualifications, and copies of certifications for the QC manager and all QC testing and inspection personnel to be used
3. Organization chart showing QC personnel and their assigned QC responsibilities
4. Example forms, including forms for certificates of compliance, hard copy test result submittals, and inspection reports
5. Methods and frequencies for performing QC procedures, including inspections and material testing
6. Procedures to control quality characteristics, including standard procedures to address properties outside of the specified operating range or limits, and example reports to document nonconformances and corrective actions taken
7. Procedures for verifying:
 - 7.1. Materials are properly stored during concrete batching operations
 - 7.2. Batch plants have the ability to maintain the concrete consistency during periods of extreme heat and cold
 - 7.3. Admixture dispensers deliver the correct dosage within the accuracy requirements specified
 - 7.4. Delivery trucks have a valid National Ready Mixed Concrete Association certification card
8. Procedures for verifying that the weighmaster certificate for each load of concrete shows:
 - 8.1. Concrete as batched complies with the authorized concrete mix design weights
 - 8.2. Moisture corrections are being accurately applied to the aggregates
 - 8.3. Cementitious materials are from authorized sources
 - 8.4. Any water that is added after batching at the plant
9. Procedures for visually inspecting the concrete during discharge operations

Allow the Department 5 business days to review an amended QC plan or an addendum to the QC plan.

90-1.01D(10)(c) Quality Control Manager

Assign a QC manager. The QC manager must have one of the following qualifications:

1. Civil engineering license in the State
2. ACI Concrete Laboratory Testing Technician, Level 1 certification
3. NICET Level II concrete certification
4. ICC Reinforced Concrete Special Inspector certification
5. ASQ Certified Manager of Quality/Organizational Excellence with the qualifying 10 years of experience and body of knowledge in the field of concrete

During concrete placement, the QC manager must be at the plant or job site within 3 hours of receiving notification from the Engineer.

90-1.01D(10)(d) Quality Control Testing Frequencies

For each mix design used to produce CIP structural concrete, perform sampling and testing in compliance with the following tables:

Aggregate QC Tests

Quality characteristic	Test method	Minimum testing frequency
Aggregate gradation	California Test 202	Once per each day of pour
Sand equivalent	California Test 217	
Cleanness value	California Test 227	
Moisture content of fine aggregate	California Test 226	1–2 times per each day of pour, depending on conditions

Concrete QC Tests

Quality characteristic	Test method	Minimum testing frequency
Slump	ASTM C143/C143M	Once per 100 CY or each day of pour, whichever is more frequent, and when requested by the Engineer
Uniformity ^a	ASTM C143/C143M, California Test 533, and California Test 529	When ordered by the Engineer
Air content, (freeze-thaw area)	California Test 504 ^b	If concrete is air entrained, once per 30 CY or each day of pour, whichever is more frequent
Air content, (non-freeze-thaw area)	California Test 504 ^b	If concrete is air entrained, once per 100 CY or each day of pour, whichever is more frequent
Temperature	California Test 557	Once per 100 CY or each day of pour, whichever is more frequent
Density	California Test 518	
Compressive strength ^{c,d}	California Test 521	

^aAs specified in section 90-1.01D(4)

^bUse ASTM C173/C173M for lightweight concrete.

^cMark each cylinder with the Contract number, the date and time of sampling, and the weighmaster certificate number.

^dYou may need additional test samples to facilitate your schedule.

90-1.01D(10)(e) Inspection Reports

Document each inspection performed by a QC inspector in an inspection report that includes:

1. Contract number
2. Mix design number
3. Date and time of inspection
4. Plant location
5. Concrete placement location
6. Batch number
7. Reviewed copies of weighmaster certificates
8. Description of the inspection performed
9. Name, certification number, and signature of the QC inspector

90-1.01D(10)(f) Rejection of Material

If any of the QC concrete test results fail to comply with the specified requirements, the batch of concrete must not be incorporated in the work. Notify the Engineer. Repeat the QC concrete tests on each subsequent batch until the test results comply with the specified requirements.

If 3 consecutive batches fail to comply with the specified requirements, (1) revise concrete operations as necessary to bring the concrete into compliance and (2) increase the frequency of QC testing. The revisions must be authorized before resuming production. After production resumes, you must receive authorization before returning to the QC testing frequency authorized in the QC plan.

90-1.01D(11) Department Acceptance

The Department accepts concrete incorporated into CIP structural concrete members based on only the Department's test results. QC test results will not be used for Department acceptance.

Replace the table in section 90-1.02G(6) with:

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Type of work	Nominal		Maximum	
	Penetration	Slump	Penetration	Slump
	(in)	(in)	(in)	(in)
Concrete pavement	0–1	--	1.5	--
Nonreinforced concrete members	0–1.5	--	2	--
Reinforced concrete structures with:				
Sections over 12 inches thick	0–1.5	1–3	2.5	5
Sections 12 inches thick or less	0–2	1–4	3	6
Concrete placed under water	--	6–8	--	9
CIP concrete piles	2.5–3.5	5–7	4	8

Replace the introductory clause of the 6th paragraph of section 90-1.02H with:

04-19-19

For pavement, the total cementitious material must be composed of one of the following options, by weight:

Add after the 6th paragraph of section 90-1.02H:

04-19-19

For structures, the total cementitious material must be composed of one of the following options, by weight:

1. 25 percent natural pozzolan or fly ash with a CaO content of up to 10 percent and 75 percent portland cement.
2. 20 percent natural pozzolan or fly ash with a CaO content of up to 10 percent, 5 percent silica fume, and 75 percent portland cement.
3. 12 percent silica fume, metakaolin, or UFFA, and 88 percent portland cement.
4. 50 percent GGBFS and 50 percent portland cement.
5. 25 to 50 percent fly ash with a CaO content of up to 10 percent, and no natural pozzolan. The remaining portion of the cementitious material must be portland cement or a combination of portland cement and UFFA, metakaolin, GGBFS, or silica fume.

Replace section 90-1.03B(2) with:

04-19-19

90-1.03B(2) Water Method

The water method must consist of keeping the concrete continuously wet by applying water for a curing period of at least 7 days after the concrete is placed.

Keep the concrete surface wet by applying water with an atomizing nozzle that forms a mist until the surface is covered with curing media. Do not allow the water to flow over or wash the concrete surface. At the end of the curing period, remove curing media.

Use any of the following curing media to retain moisture:

1. Mats, rugs, or carpets
2. Earth or sand blankets
3. Sheeting materials complying with the durability and water vapor transmission rate specified in section 5 of ASTM C171

To ensure proper coverage during curing:

1. Cover the entire concrete surface with the curing media

